

Joint Meeting BGA and CFMS

Friday 2<sup>nd</sup> December 2011

## **OBSERVATIONAL METHOD**

**Use of “review” and “back analysis” to  
implement the “Best Way Out” approach.**

**By**

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- **“Best way out” processes**
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  - Review and back analysis
- **Basement Case Histories**
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  - **Canary Wharf Crossrail Station**

# History - Key dates for UK

- 40 to 60's** - **Terzaghi and Peck**
- 1969** - **Peck's Rankine Lecture**
- Early 1990's - Channel Tunnel, Limehouse Link Projects
- 1994** - **Geotechnique Symposium in Print**
- 1995** - **EC7 OM Clause**
- 1996 - ICE and HSE - NATM publications
- 1999** - **CIRIA - OM Report No 185**
- 2001 - ICE Managing Geotechnical Risk
- 2003 - Ciria C580 – Embedded retaining Walls.
- 2006 - Geotechnet - [www.geotechnet.org](http://www.geotechnet.org)

# Peck's (1969) Observational Method – Eight Ingredients

1. **Sufficient SI** to establish general nature / properties of deposits.
2. Assess **Most Probable** and **Most Unfavourable** conditions.
3. Establish **Design** based on **Most probable**.
4. Select **Monitoring parameters** and **calculate values**.
5. Calculate values for **most unfavourable** conditions.
6. Select design **modification options**.
7. **Monitor** and **evaluate** actual conditions.
8. **Modify** design to suit actual conditions.



# Peck (1969) OM applications

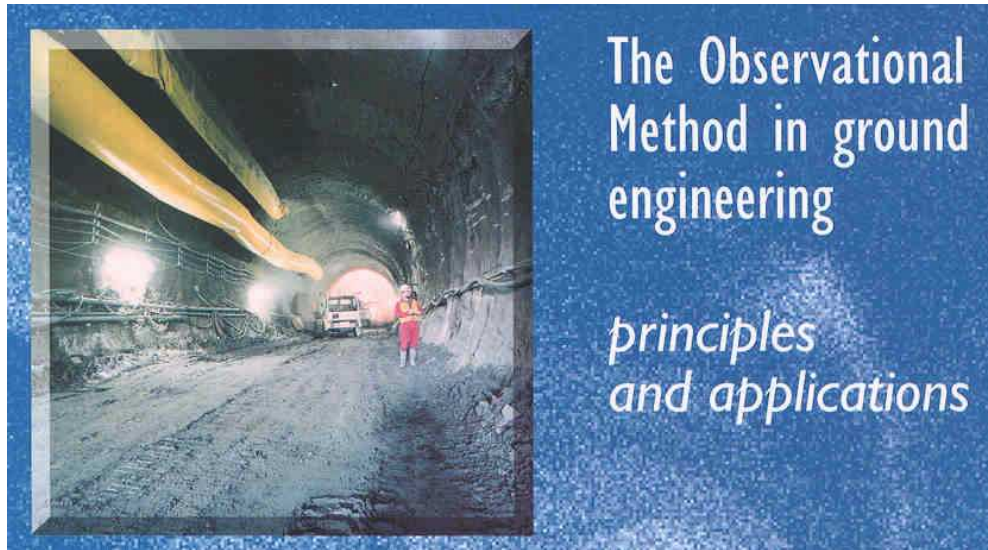
**“Ab Initio” OM** - planned from start of work

- Harris Bank – Chicago strut monitoring
- Bay Transit Tunnels – Volume loss

**“Best way out” OM** – introduced during work

- Cleveland Ore Terminal - soft clays – stockpiles of iron ore
- Cape Kennedy Causeway – Hydraulic fill

# Ciria (1999) Report 185 - Nicholson, Tse and Penny



## Goals

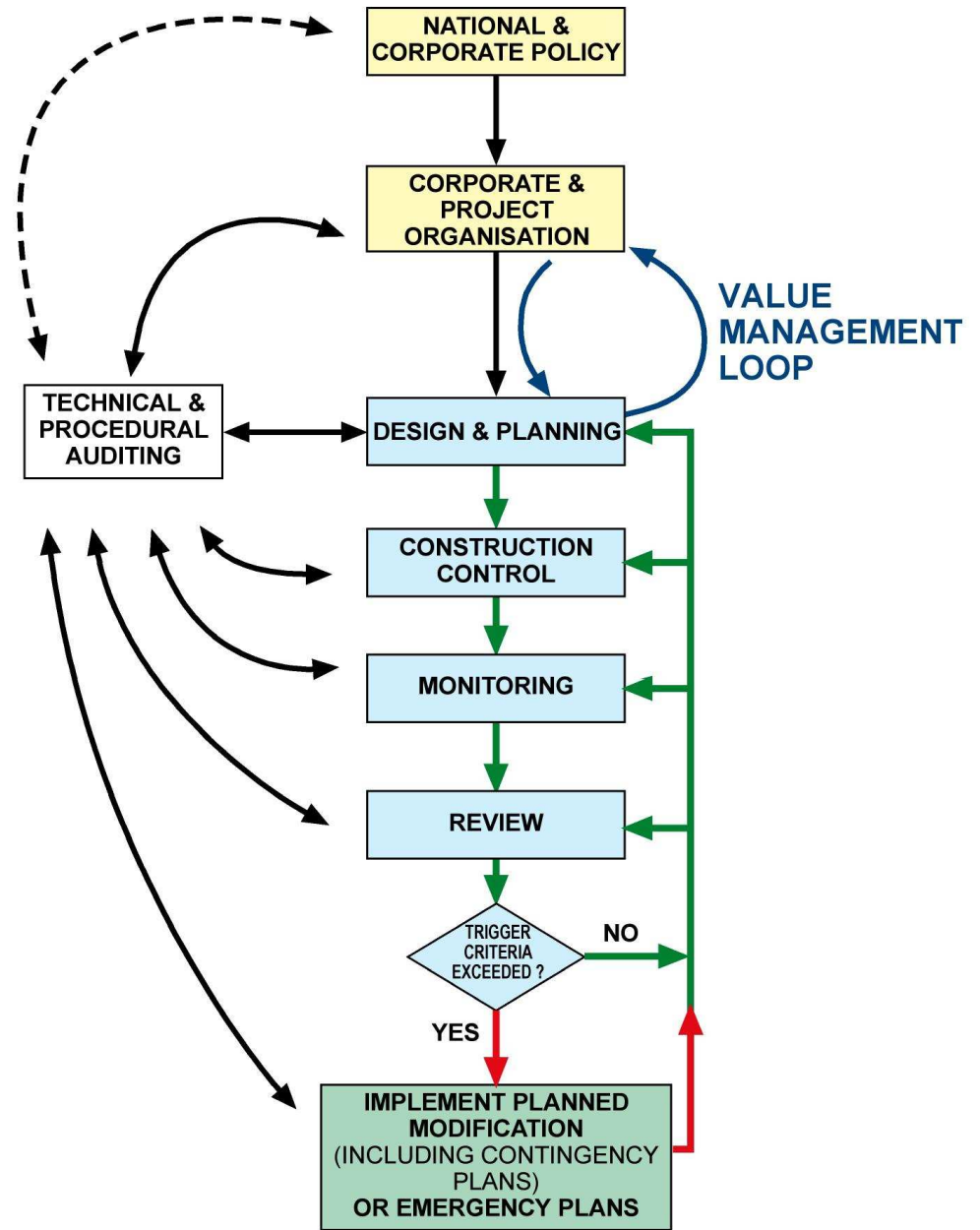
- Clarify OM definition and process
- Integrate OM process into modern design
- Focus on “Ab Initio” applications – better planning

# CIRIA (1999) - OM Definition

- The Observational Method in ground engineering is a **continuous, managed, integrated, process of design, construction control, monitoring and review** which enables **previously defined modifications** to be incorporated during or after construction as appropriate. All these aspects have to be demonstrably **robust**. The objective is to **achieve greater overall economy without compromising safety**.
- The Method can be adopted from the inception of a project or later if benefits are identified. However, the Method **should not be used where there is insufficient time** to implement fully and safely complete the **planned modification** or emergency plans.

# Ciria (1999) R185 Figure 1.2 The OM Process

Focused on Ab Initio  
OM applications



# Design Parameters - Peck's (1969) OM and Current Codes

- **Peck (1969)**

- OM conditions/values

- **Most Probable**

- **Not used**

- **Most Unfavourable**

- **UK Current Codes**

- CIRIA C580
- Eurocode – EC7

- **Not used**

- **Mod Conservative or Characteristic**

- **Worst credible**

## Predefined Design Process

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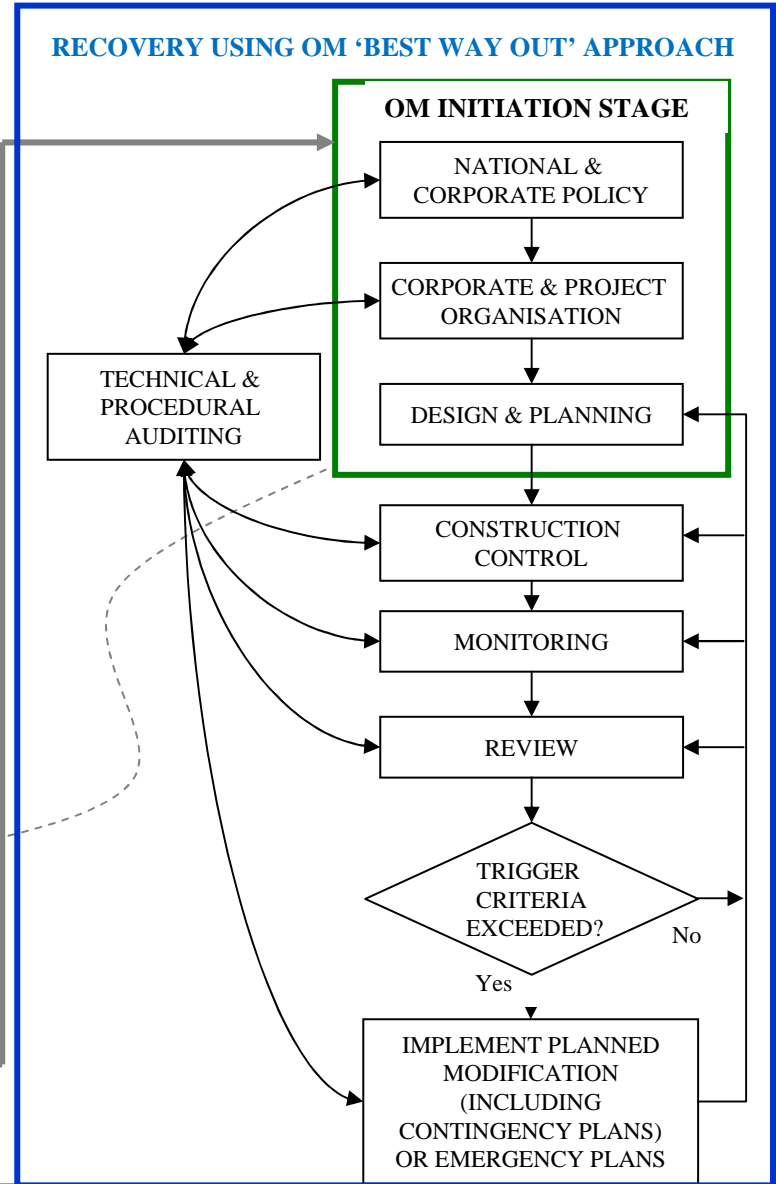
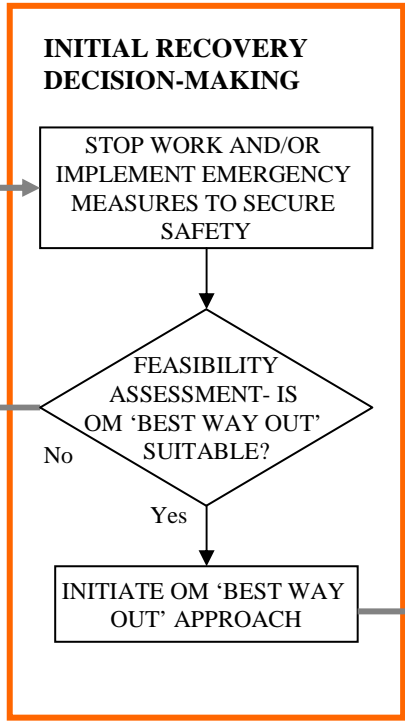
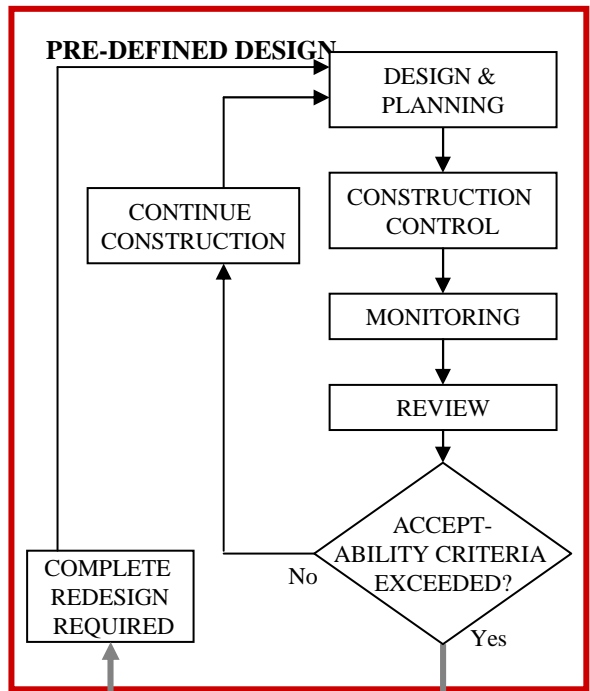
- Permanent works
- One set of parameters (MC)
- One design / predictions
- Outline construction method
  
- Trigger values
  
- Contractor's temp design /method statement
  
- Monitoring checks trigger values not exceeded
  - If exceeded Back Analyse -
  - Introduce OM - Best Way Out
  
- Emergency plan

## The OM Process - Ab Initio

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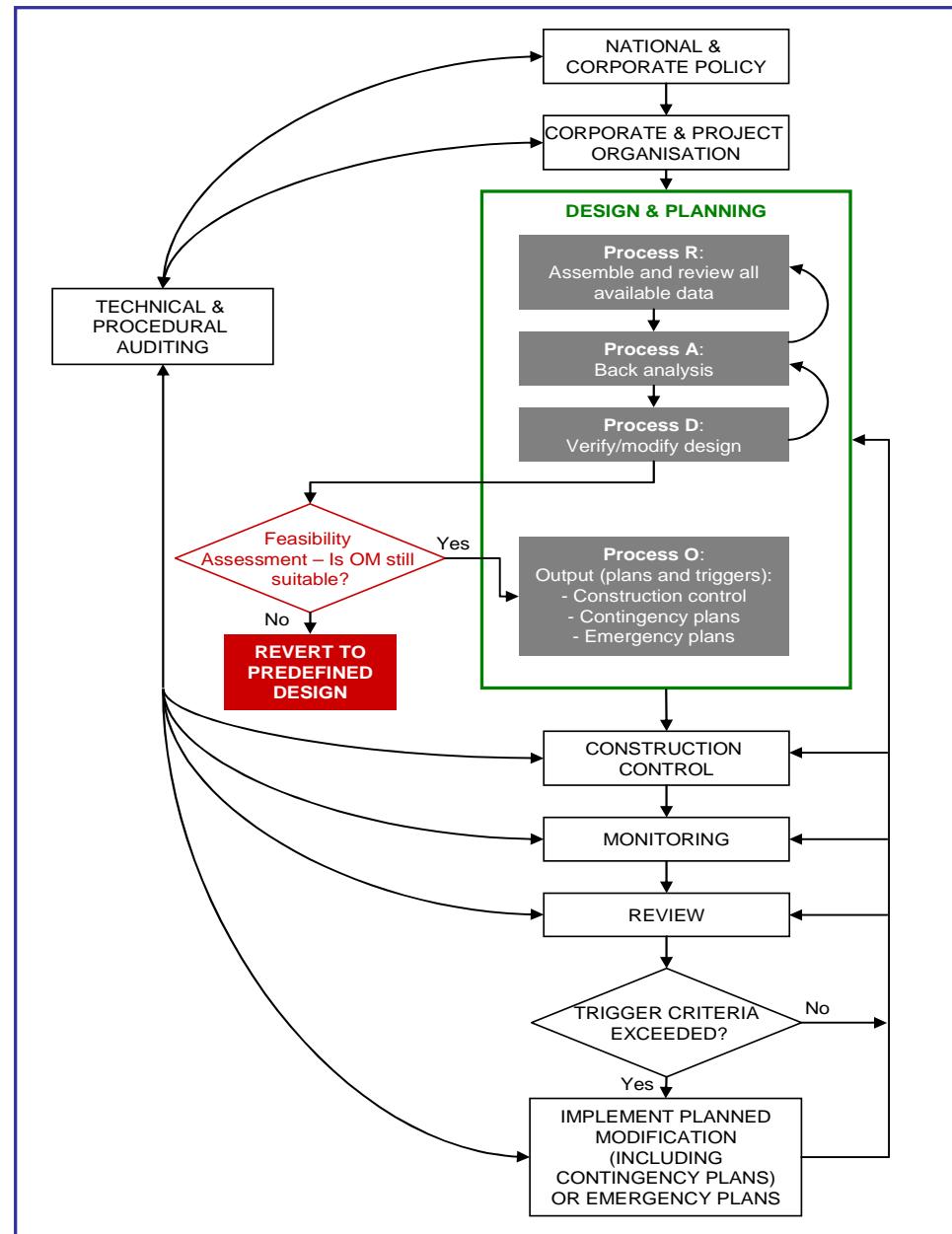
- Temporary works (mainly)
- Two sets of parameters (MC +MP)
- Two designs / predictions
- Integrated design and construction methods
- Methods relate to triggers
  
- Comprehensive and robust monitoring system
- Review and modify process
  - Contingency plan
  - Improvement plan
  
- Emergency Plan

# Change from “Pre-defined” design to “Best Way Out” approach



# Recovery using OM – “Best way Out” at “Design and Planning” Stage

- Four Processes:-
- R – Review
- A – Back Analysis
- D – Design remaining work
- O - Output

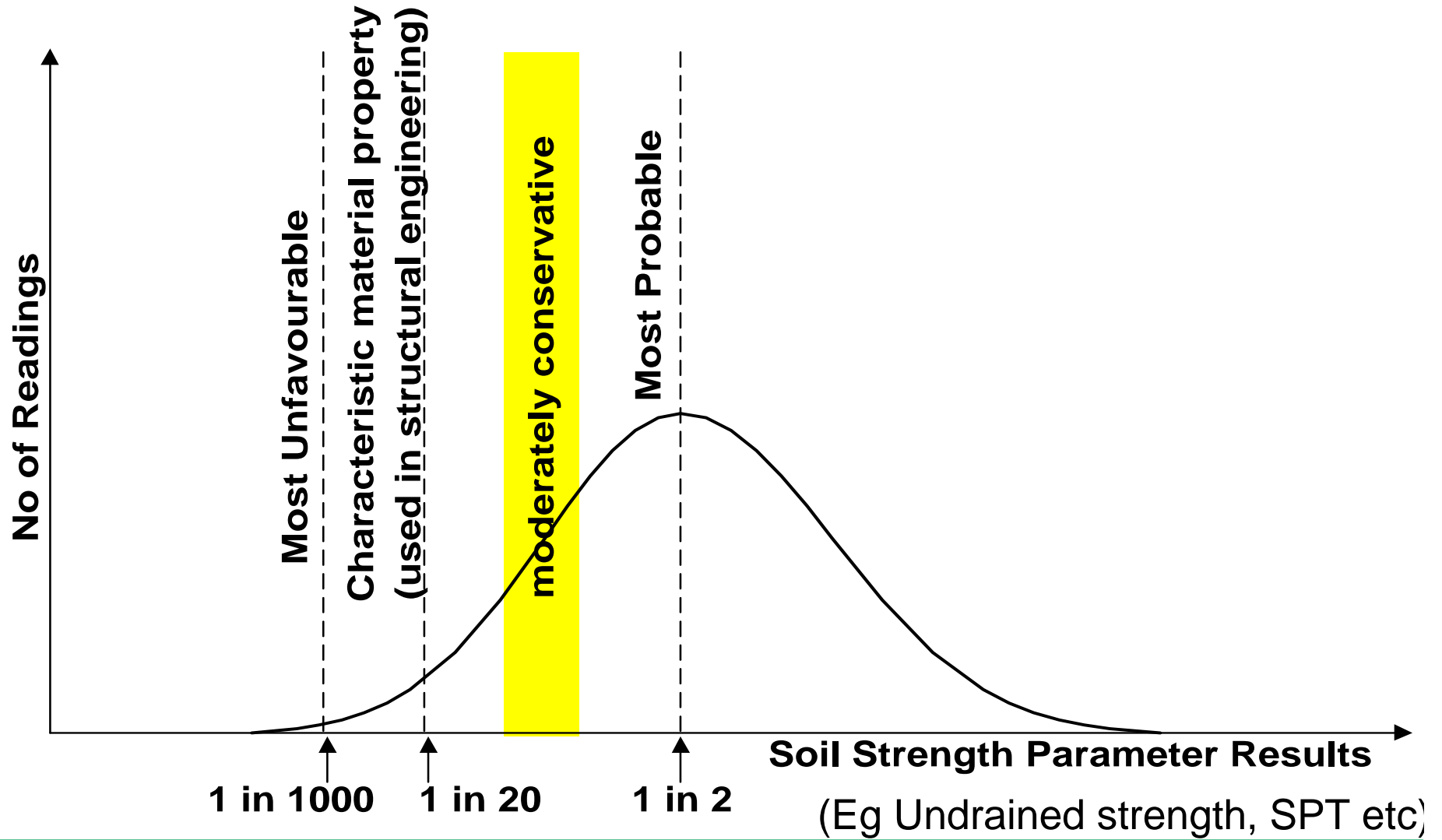




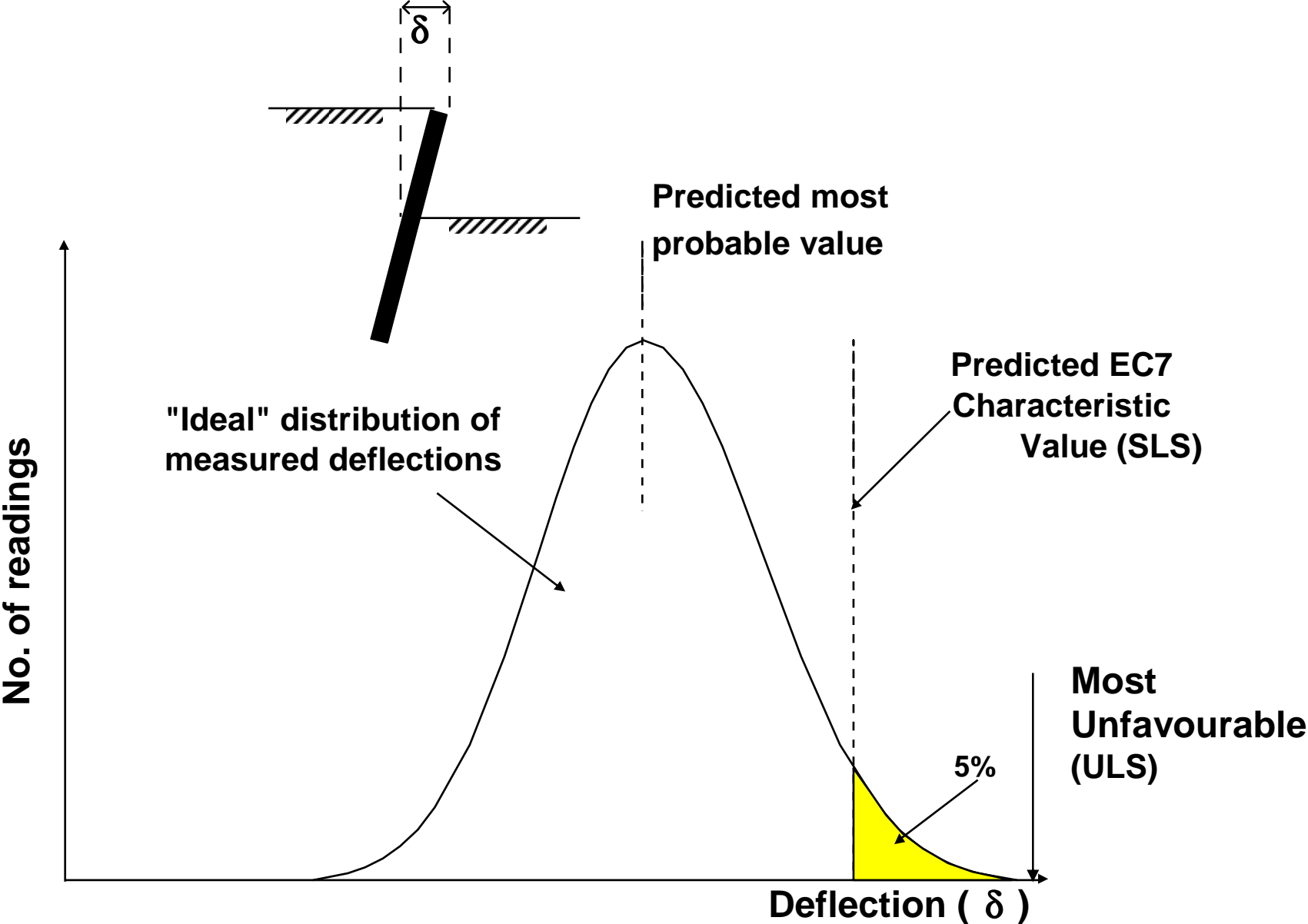
## Eurocode EC7 Cl 2.7 (1989 and 1995)

- Recognised prediction is difficult in Geotechnics –  
OM used in these cases.
  - 1) Establish limits of behaviour.
  - 2) Acceptable probability actual behaviour within limits.
  - 3) Monitoring plan, response times and contingencies.
  - 4) Contingencies adopted if real outside acceptable range.

# UK Design Codes - Soil Strength Parameters



# Ideal EC7 Predicted versus Measured Performance

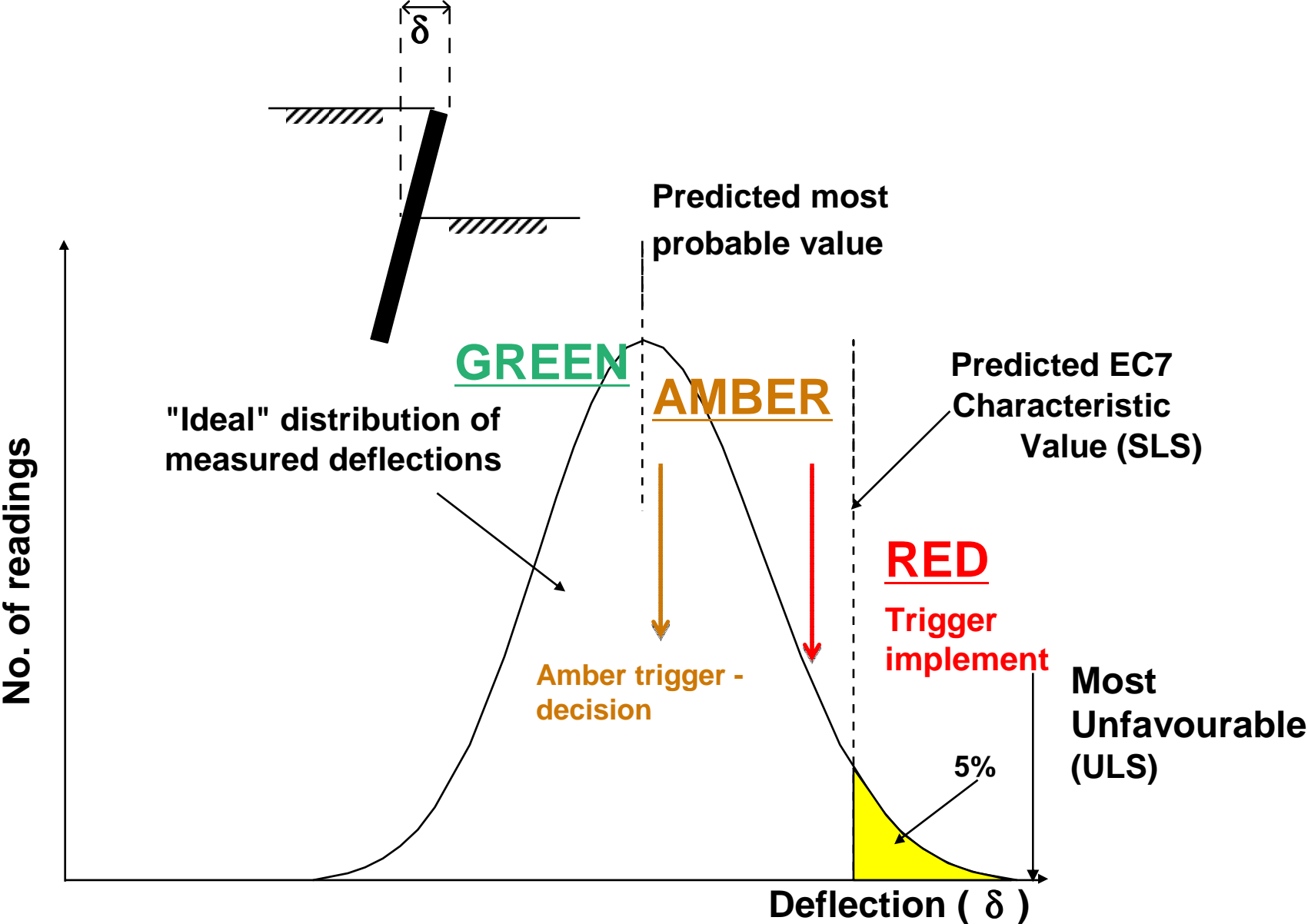


# Trigger Criteria

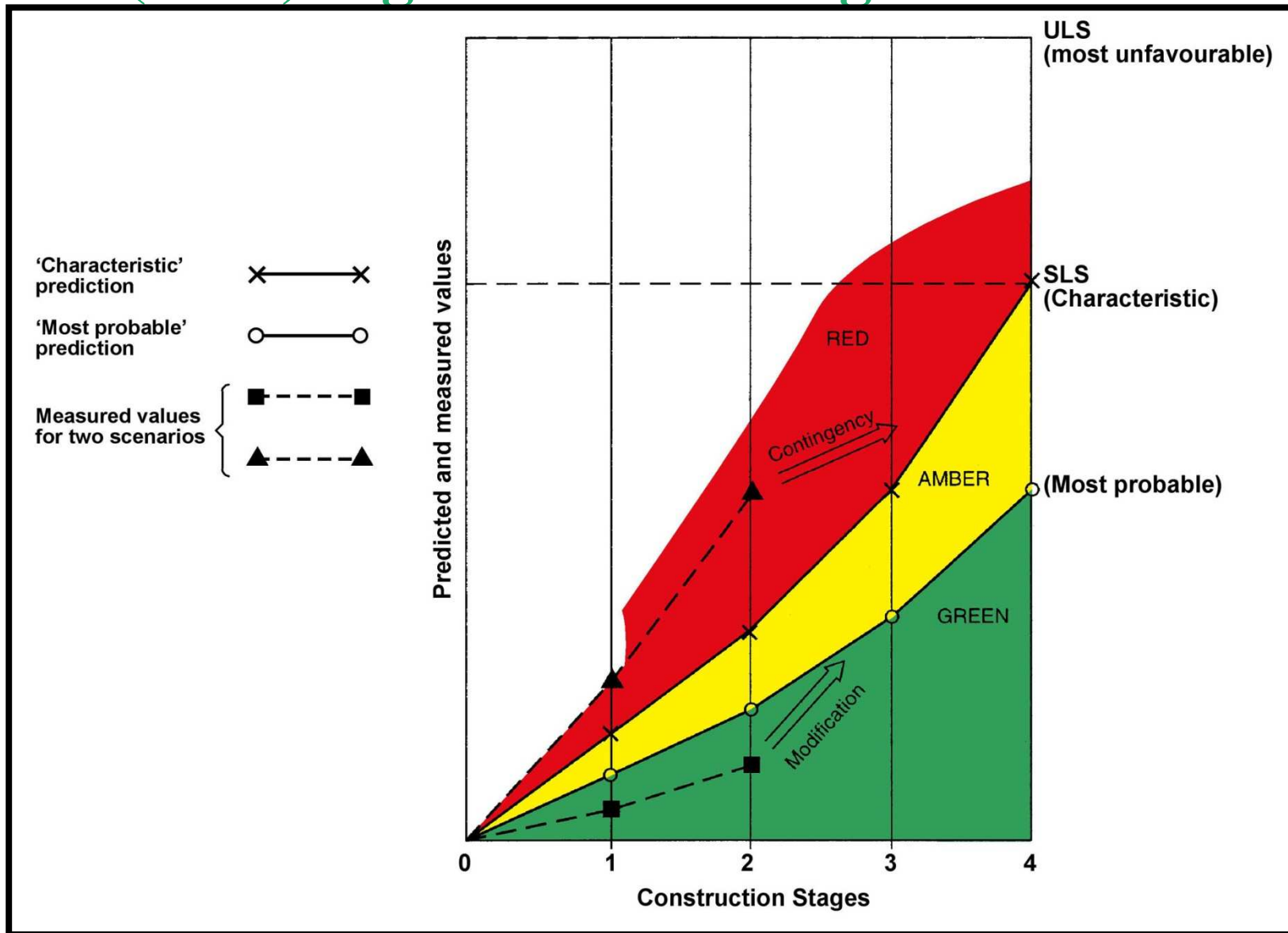
## Traffic light conditions include:-

- **Green** = Safe site condition.
- **Amber** = Decision stage
- **Red** = Implement planned modifications
- **Emergency** = Evacuation  
(Not normally part of OM. Required under CHSW Reg (1996).  
Relates to Ultimate Limit State.)

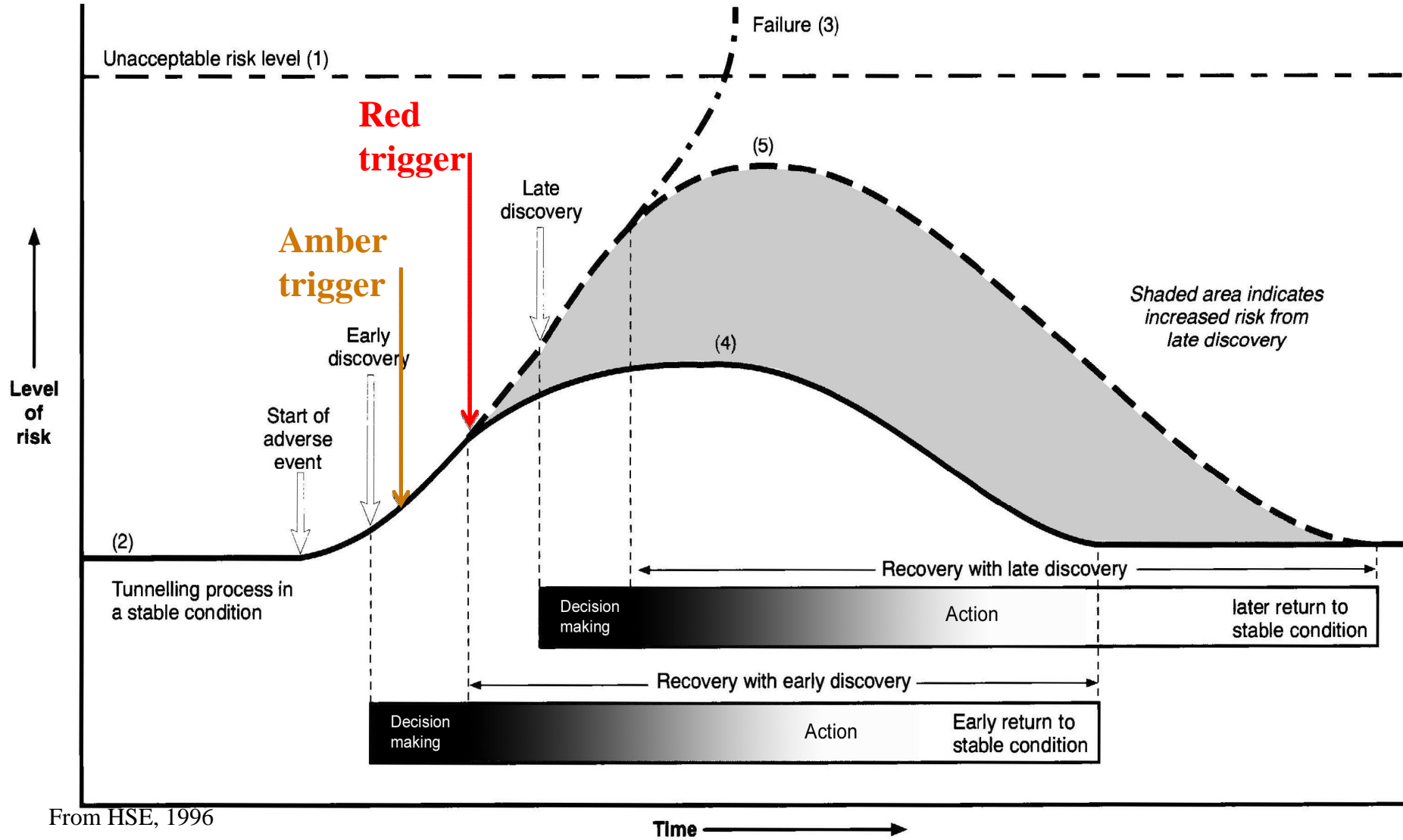
# Ideal EC7 Predicted versus Measured Performance



# Ciria (1999) Fig 3.13 Multi Stage Excavation



# HSE 'Discovery – Recovery' Model eg for tunnelling



# Case Histories

- **Kings Place - London**
- **Nicol Highway Collapse - Singapore**
- **Canary Wharf – Crossrail Station Box**
- **Donegall Quay - CFA piles**

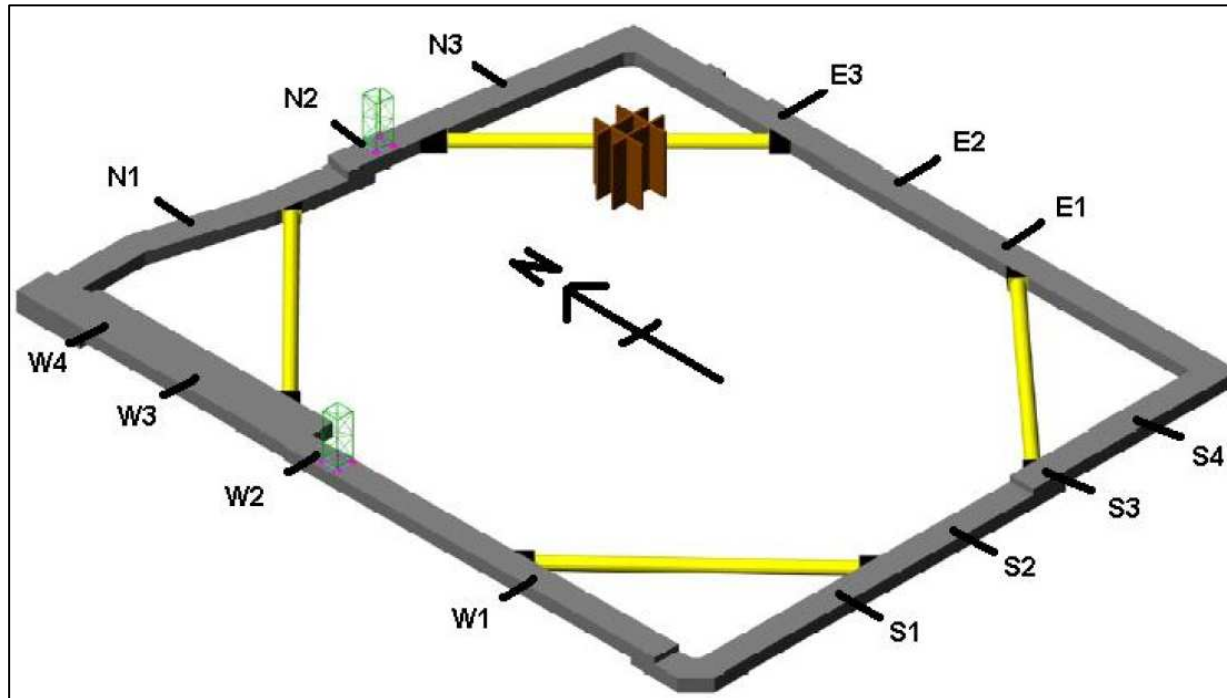


# Kings Place – OM – Ab initio

- Damage assessment trigger
  - 50mm max wall deflection
- Diaphragm Wall
  - 1.0m thick
- 1 level of temporary corner props
- 16m retained height
- Observational Method

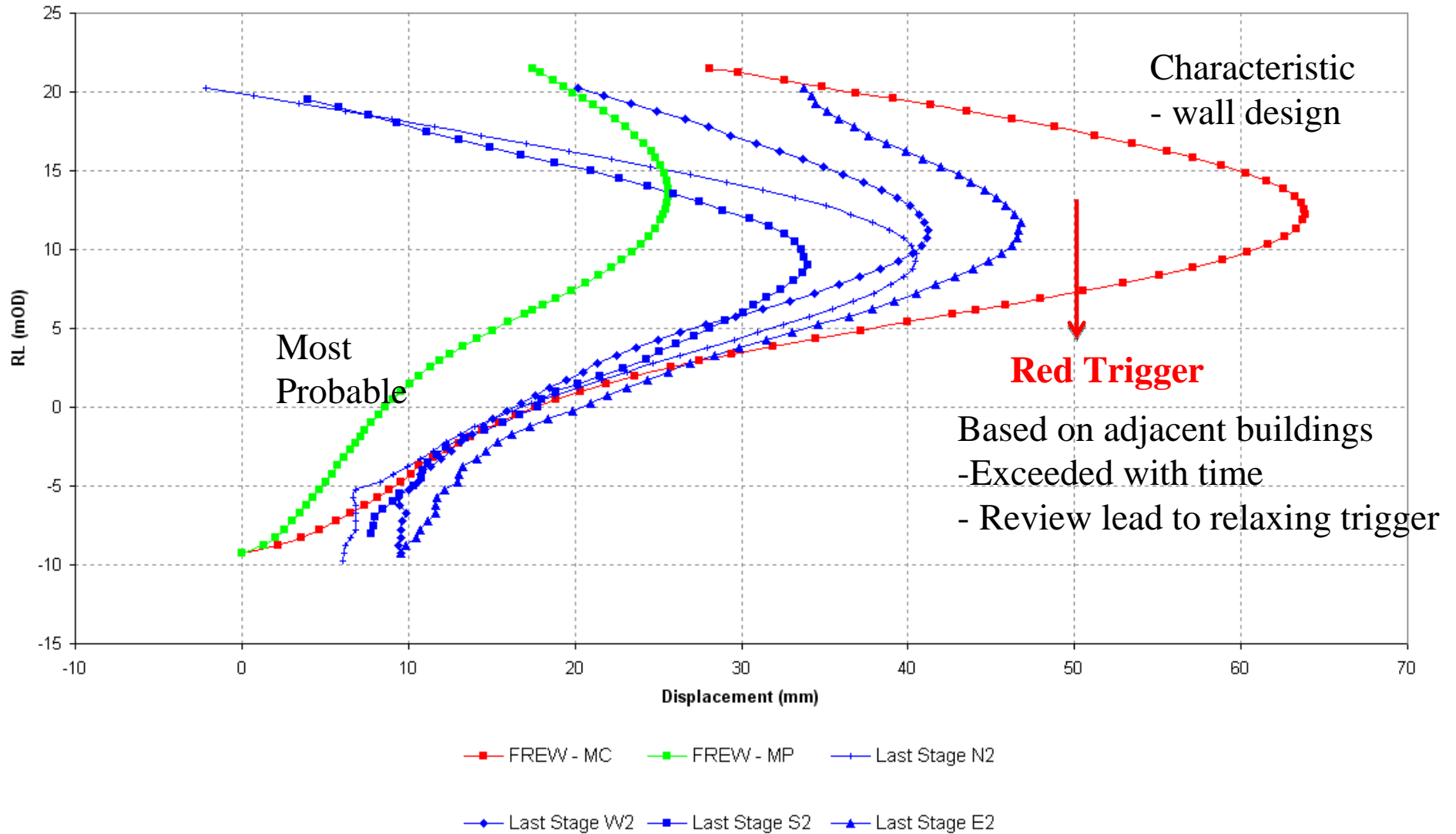


# Kings Place - Instrumentation



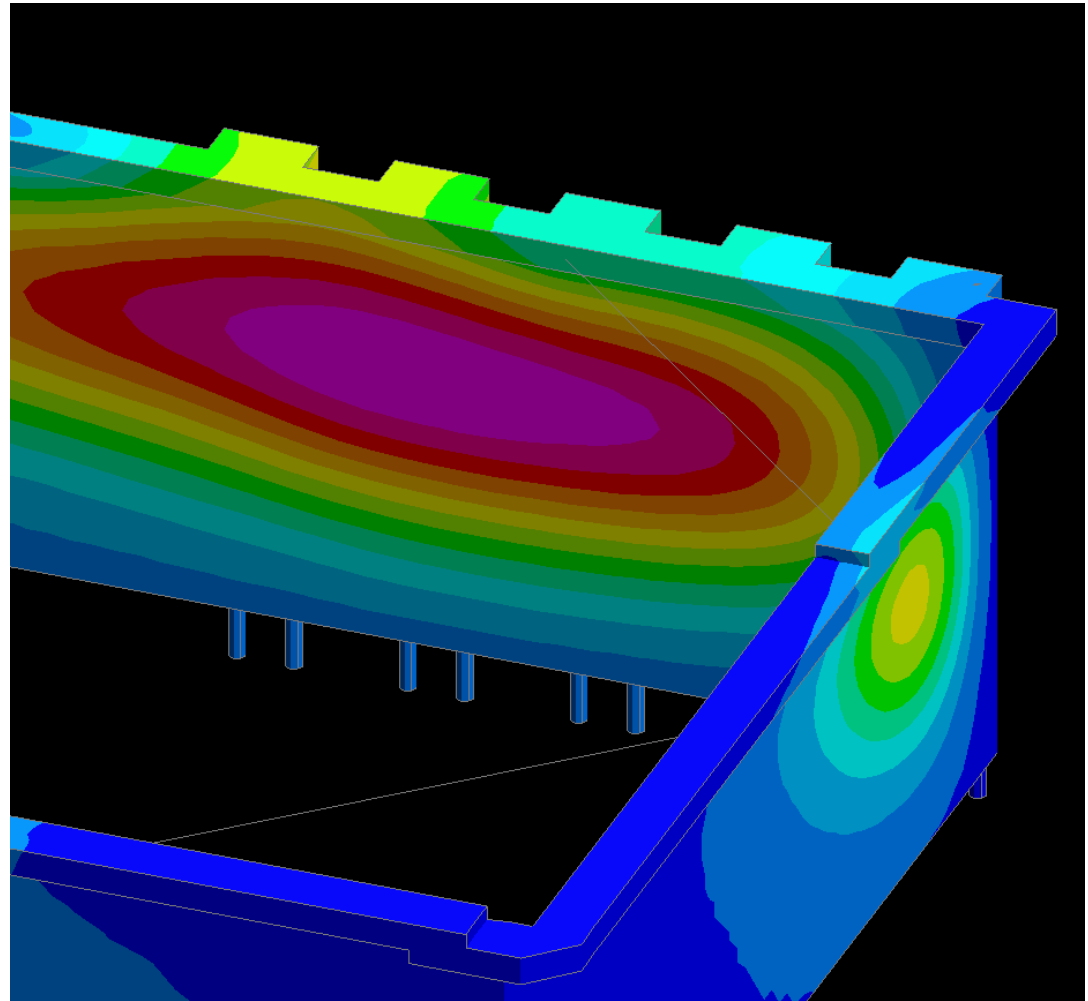
- 14no. inclinometers + 14no. survey targets
- 32no. strain gauges at props
- 40no. Ground survey stations

# Frew predictions “Last Stage” data



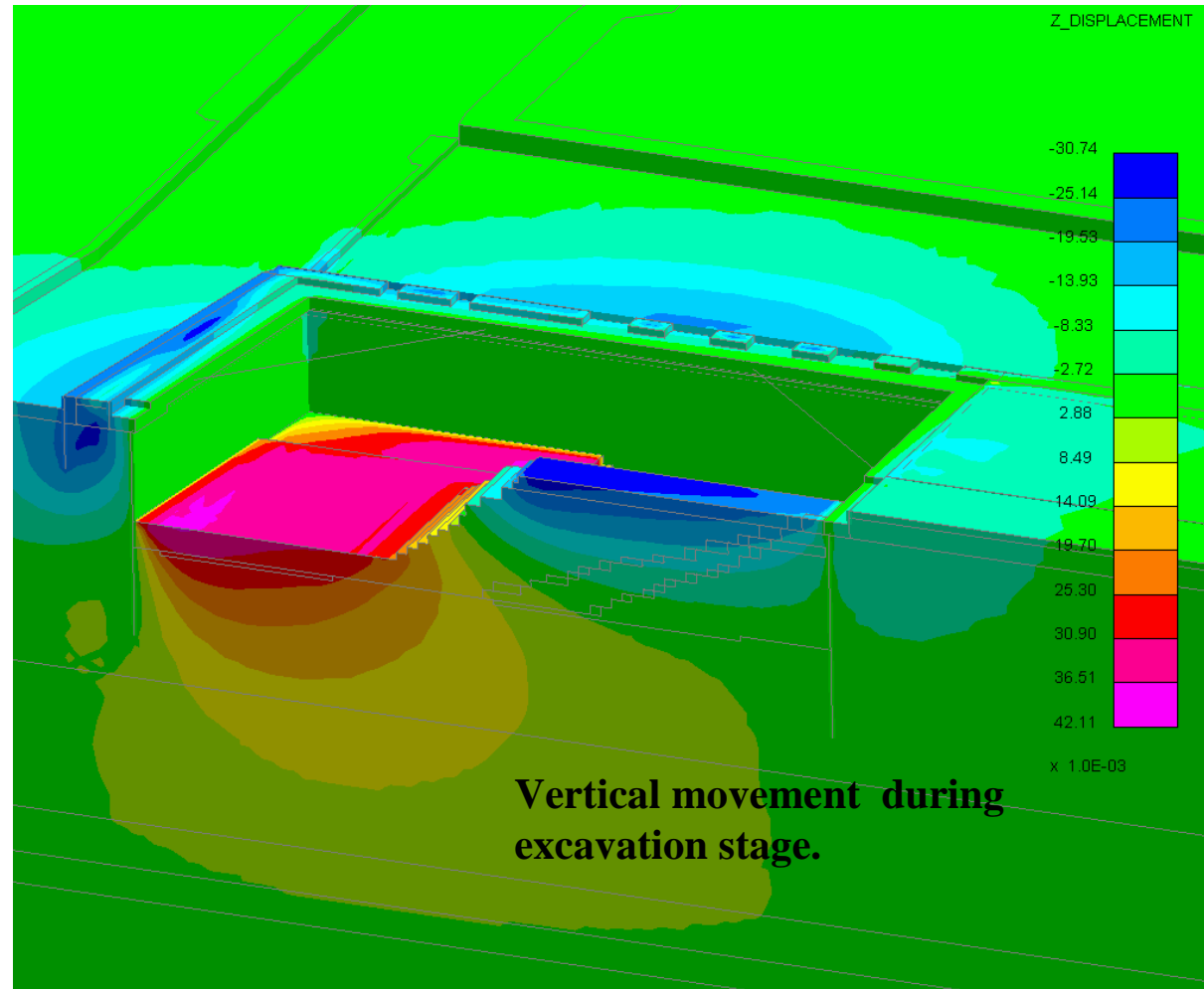
# Back Analysis - LS Dyna - 3D model

- **Small Stain Model**
- **3D geometry**
- **Assess effect of**
  - **berm excavation sequences**
  - **corner effects**
- **Soil parameters refined.**
- **Goal to assess “Characteristic” and “Most Probable” sets of parameters**
- **Monitoring data compared to numerical analyses.**
- **Refine trigger criteria for Observational Method**



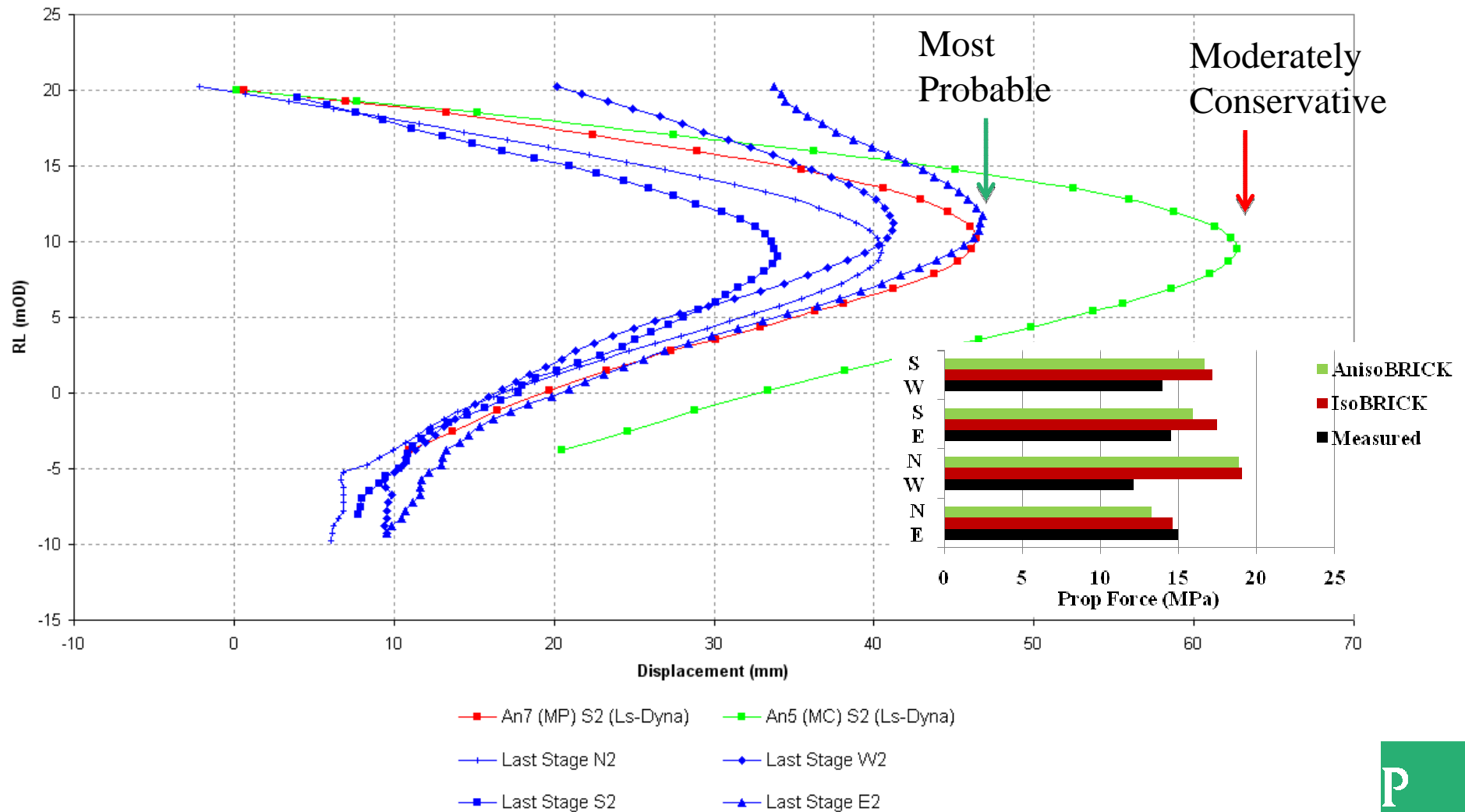
# Analysis Summary

- **Approx 600,000 elements in 32 material sets.**
- **5 Analyses varying:-**
  - Soil parameter.
  - Suction limits.
- **Stages representing 8 steps of excavation modelled.**
- **Site data compared to model data.**



# MP and MC Ls-DYNA vs. “Last Stage” data

- Monitored data set matching last stage of model sequence.
- Based on Suction limit of -100kPa. - “AnisoBRICK”
- Consolidation





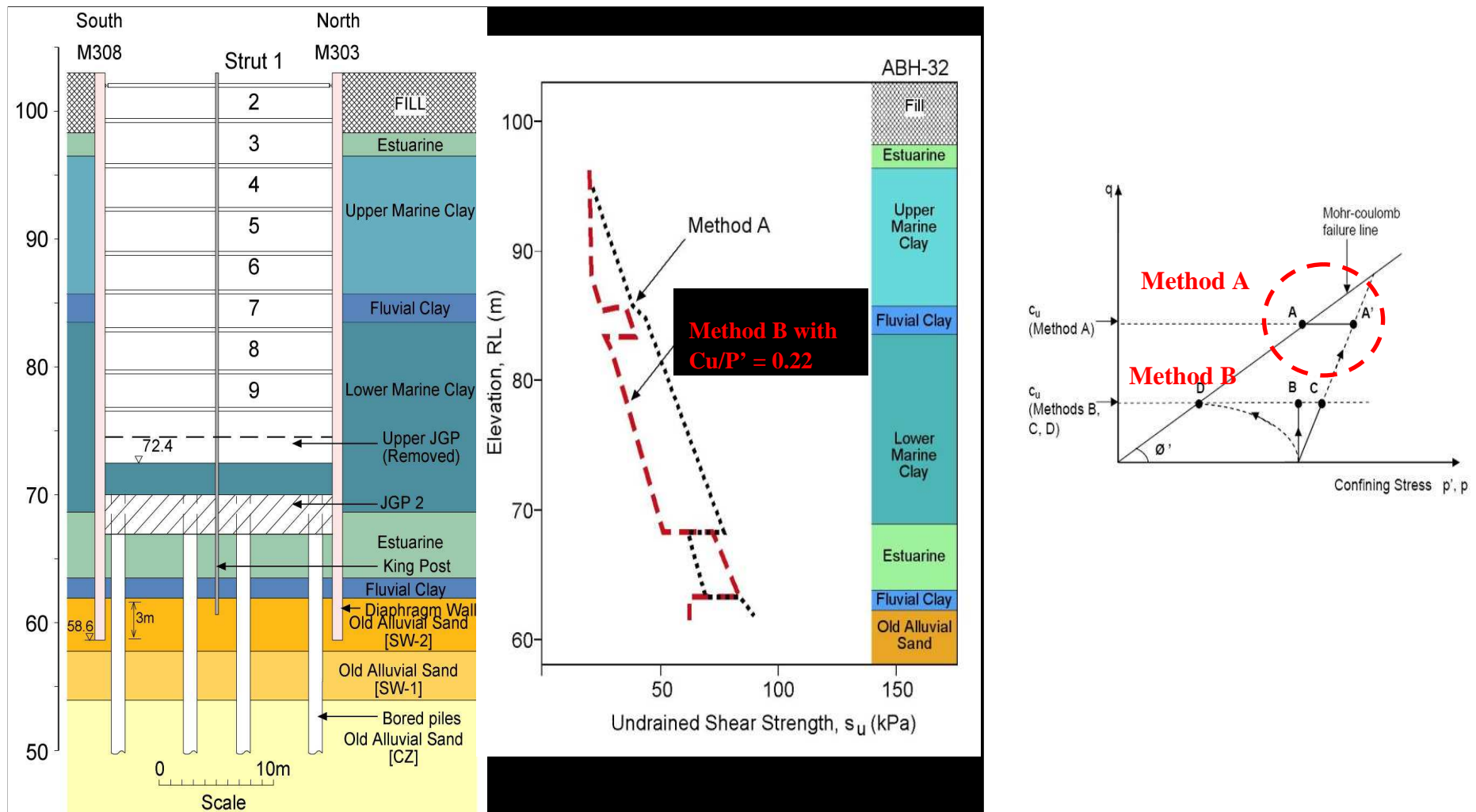
# Nicoll Highway Collapse – Not Observational Method





# Public Inquiry – Key Lessons

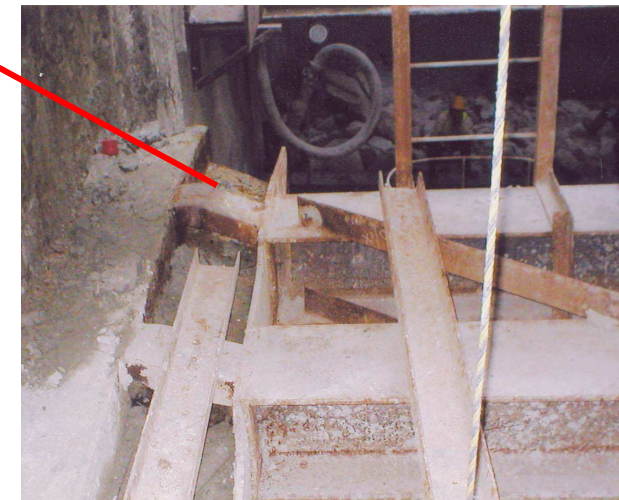
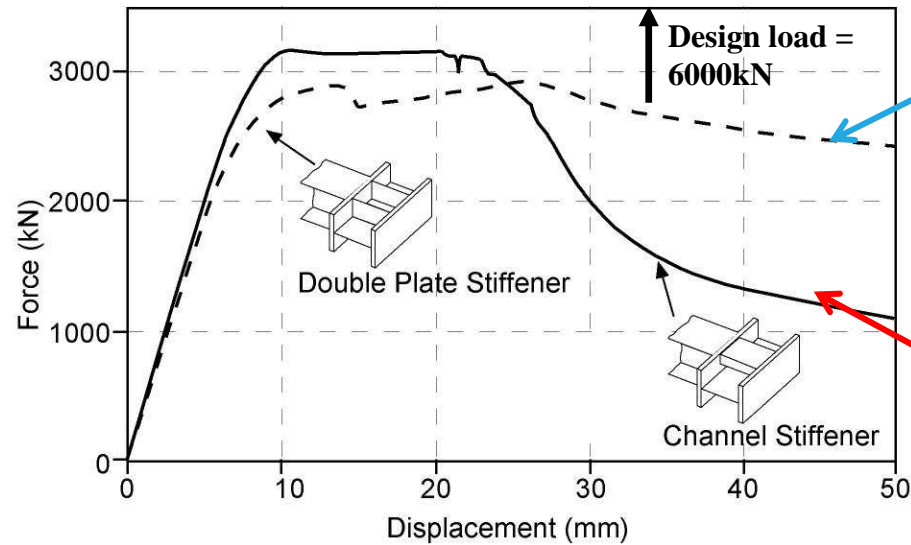
## 1. Soil model in Plaxis over estimated Marine Clay strength





# Key Lessons cont'd

## 2. Waler connection under capacity



### Many other Contributory Factors

- Monitoring and review regime – not effective
- Back analysis process – not rigorous

# Public Inquiry Lessons

## 1. Design

- **Independent check required**

## 2. Construction Quality

- **Management / Interpretation of data / instrumentation**

## 3. Contractual Arrangement

- **D and B – Production pressure**

## 4. Management/Culture

- **Effective risk management**
- **Managing uncertainties and quality**

### Comment

- **Design errors were made.**
- **Back analysis process did not pick them up properly!**

# Nicoll Highway Collapse – Implementing lessons

## Technical

- Public Enquiry – Magnus et al, (2005)
- International Conference on Deep Excavations  
28 – 30 June 2006, Singapore

## Legislation

- Building and Construction Authority (BDA)
  - Advisory Note on Deep Excavations (5-May 2005) -  
Temporary Earth Retaining Structures (TERS)
  - Updated to Advisory Note 1/09 (2-April 2009)– Earth  
Retaining or Stabilising Structures (ERSS)
  - Updated to  
Advisory

# Canary Wharf Crossrail Station – Lessons learned

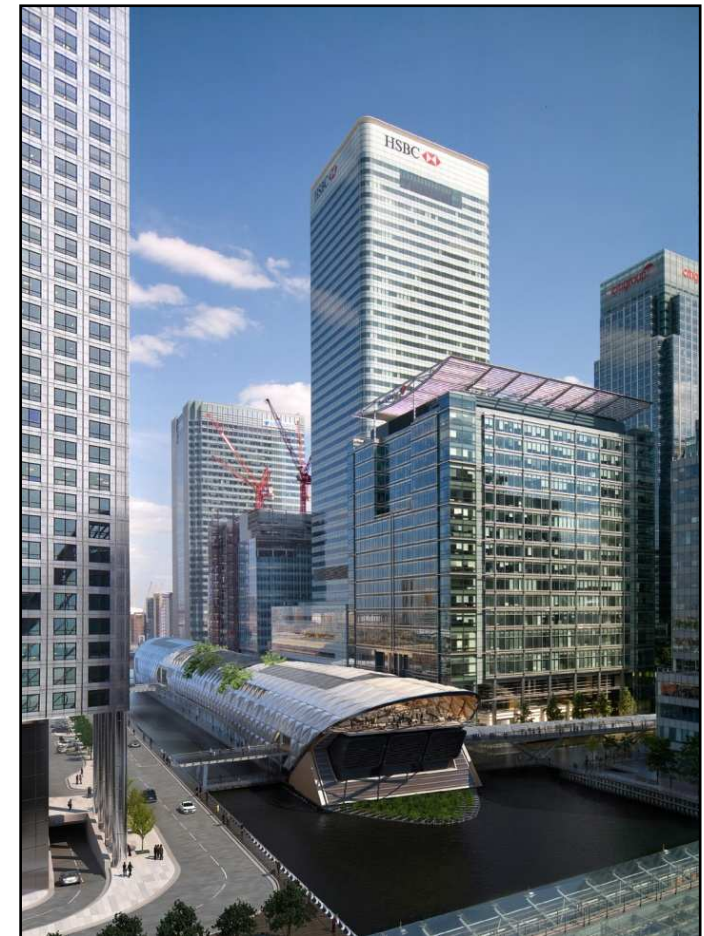
Crossrail Station at Canary Wharf

Client - CrossRail

Project Manager – Canary Wharf  
Contractors Limited

Main Contractor – Laing O'Rourke

- Geotechnical risks
- Conventional design – with triggers
- Review process.
- OM Ab Initio modification on final dig stage
- Monitoring – Exceeding Triggers



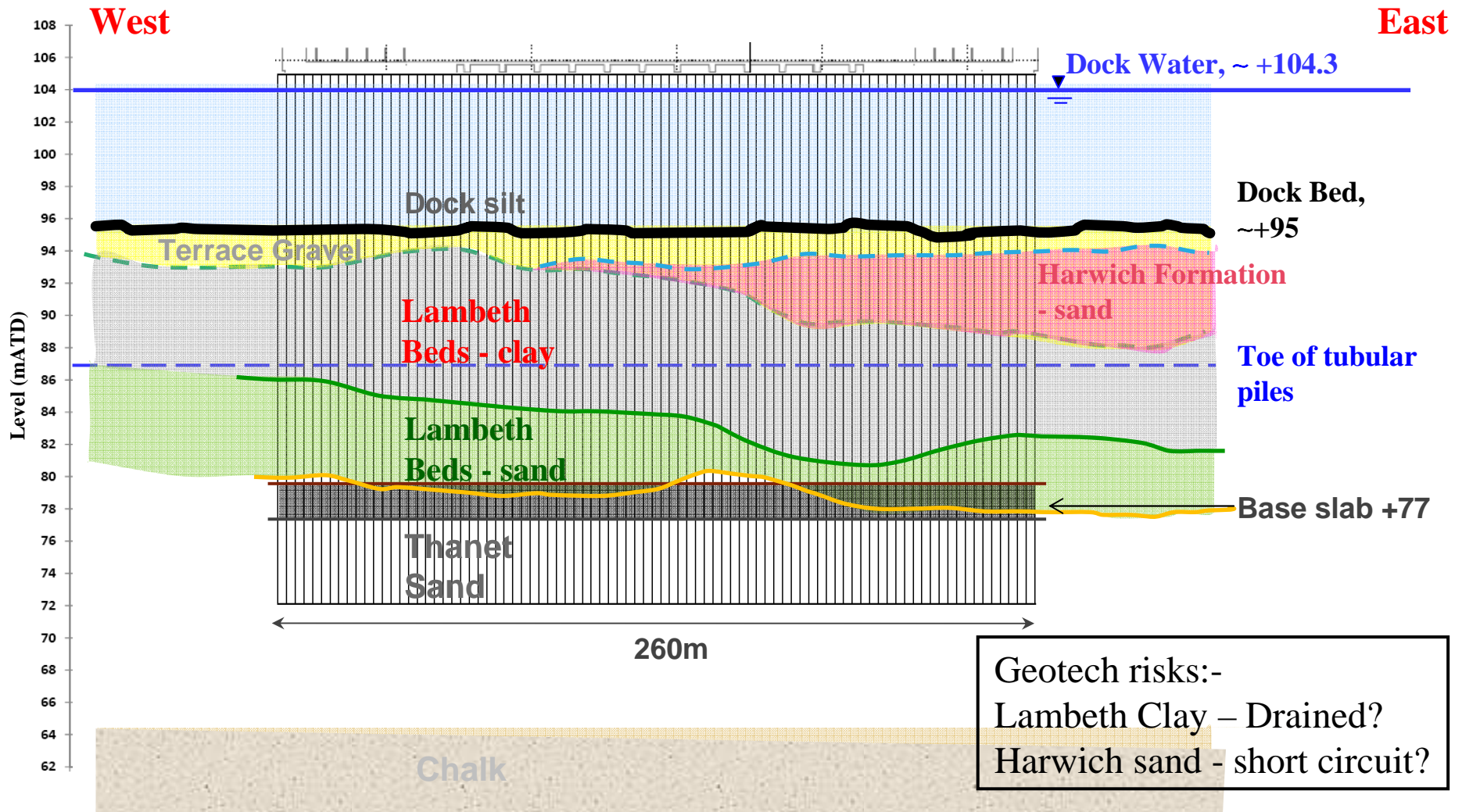


# Canary Wharf Crossrail Station Layout



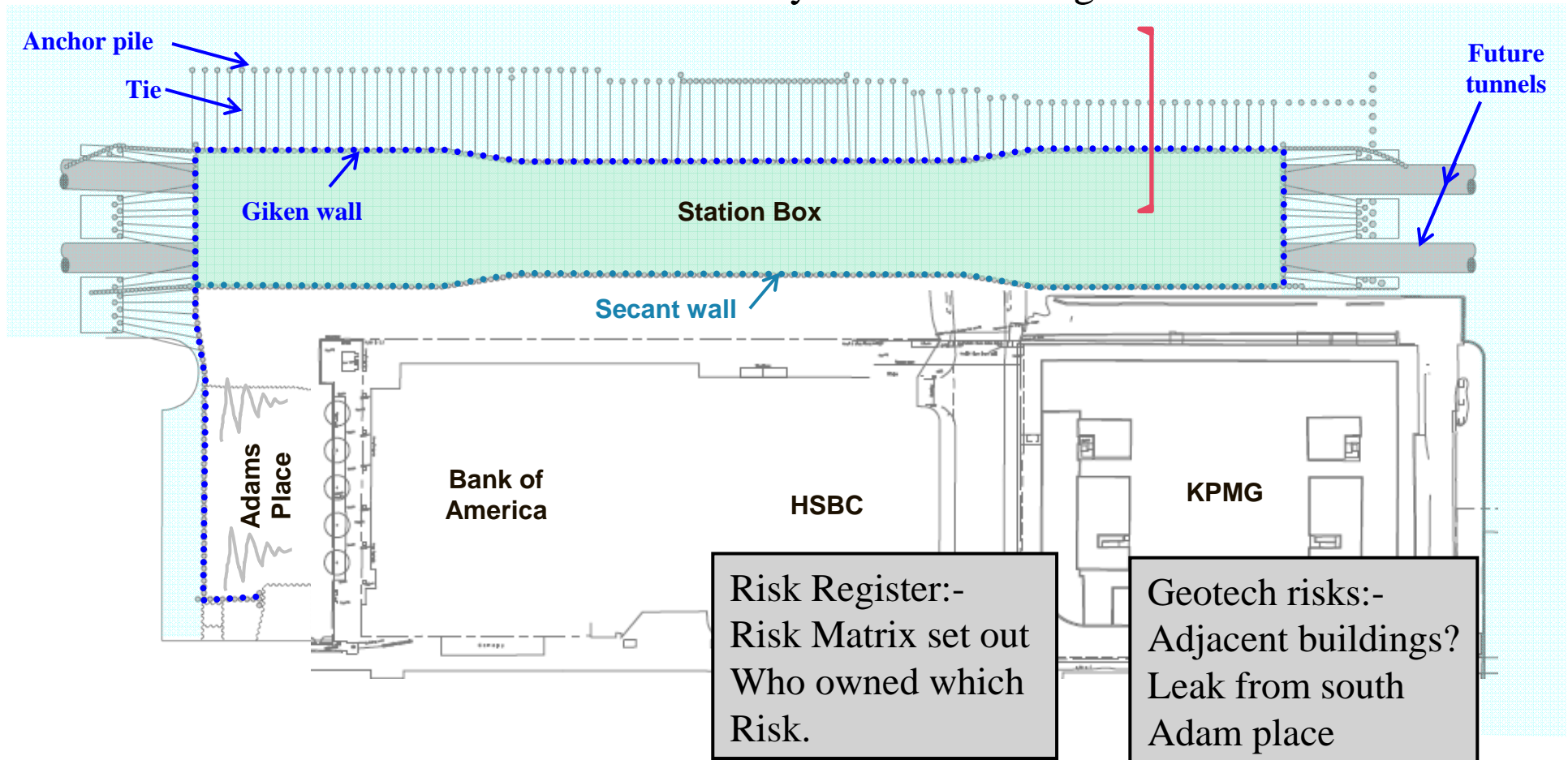
Geotech risks:-  
Adjacent buildings  
Dock structures  
DLR

# Geology along the station box



# Working with stakeholders – Final scheme evolved from many inputs

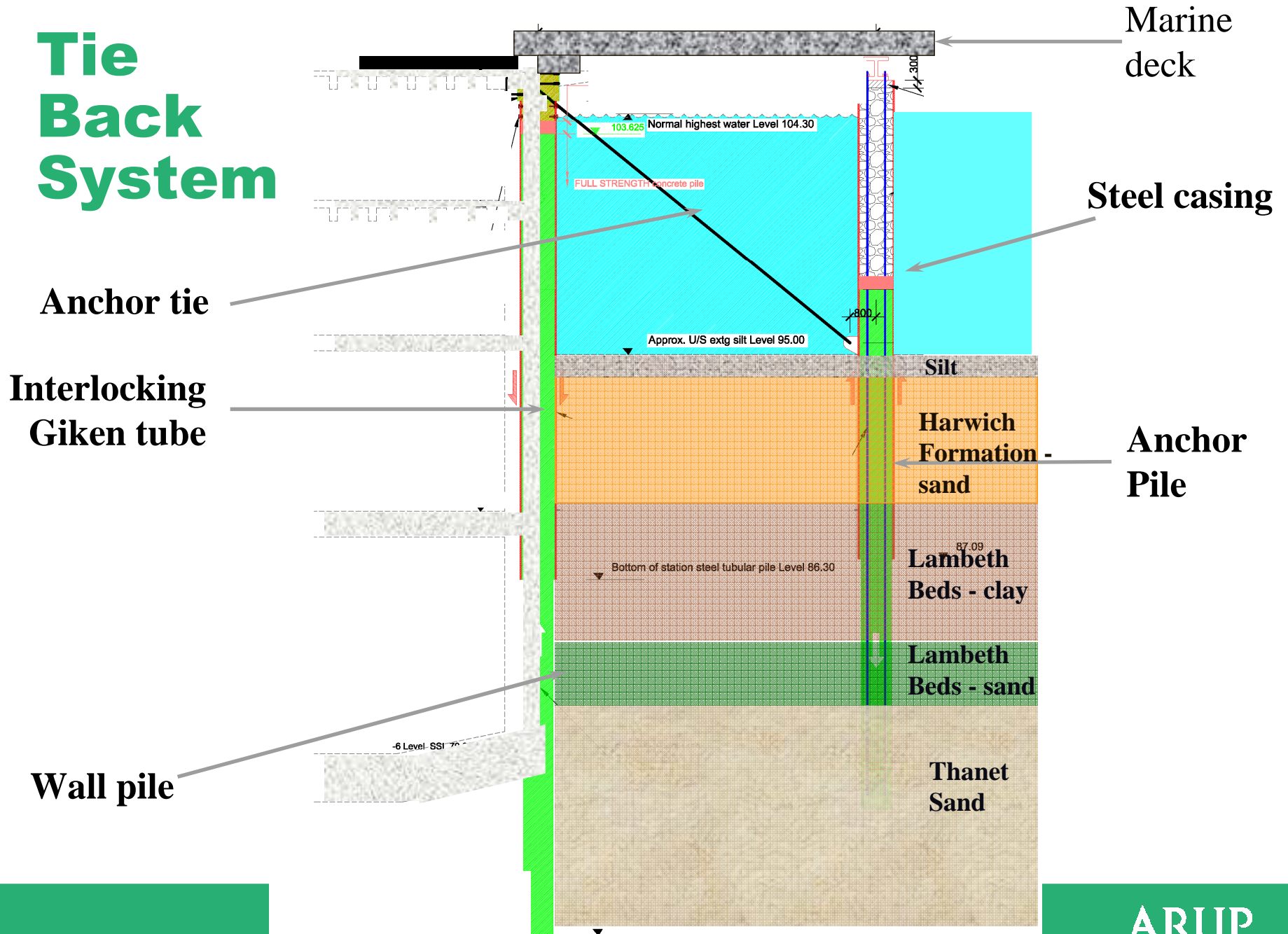
- Construction in a drained dock - asymmetric loading



Two types of retaining wall – tied back and bermed cantilever walls



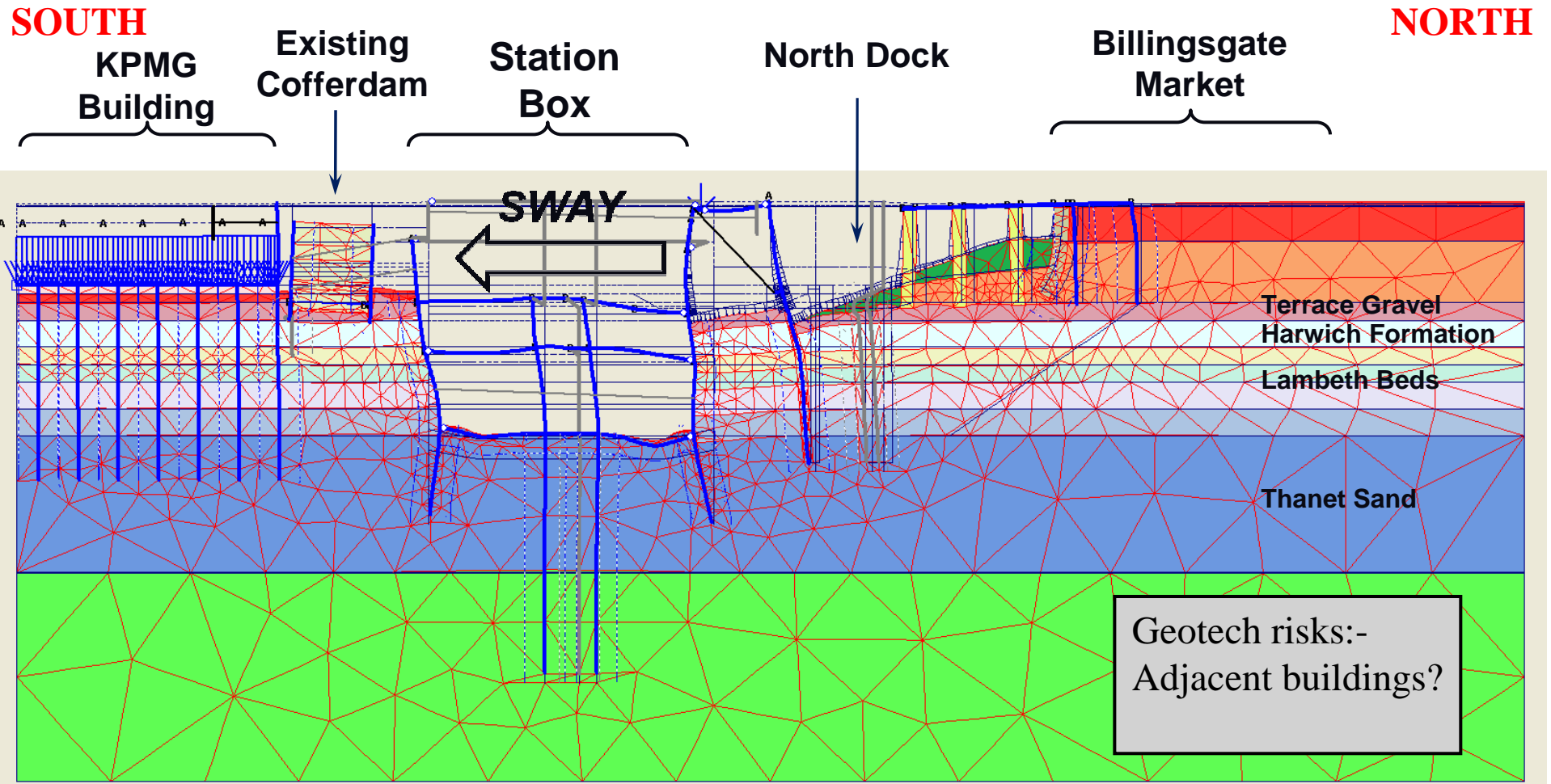
# Tie Back System





# Soil-structure interaction – finite element model

## Exaggerated Plaxis displacement plot



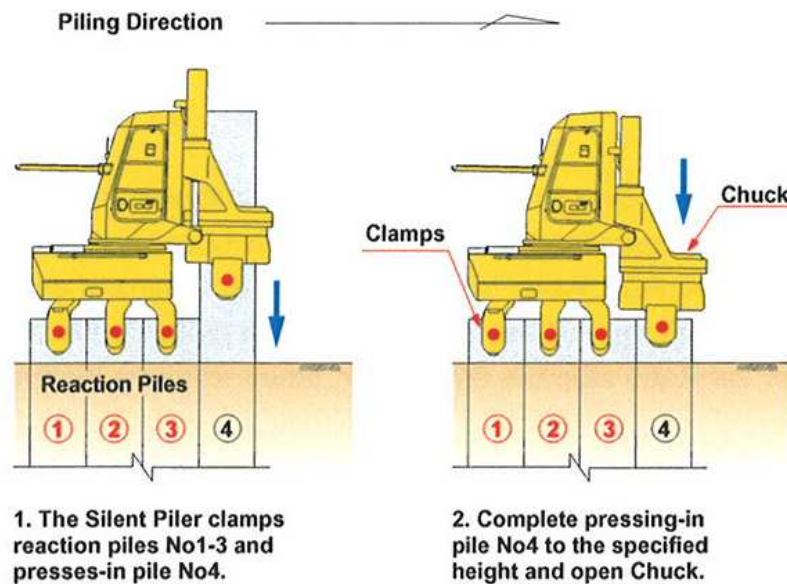
Finite element method – capturing out-of-balance loading (sway) and ground movement

# Giken push in process

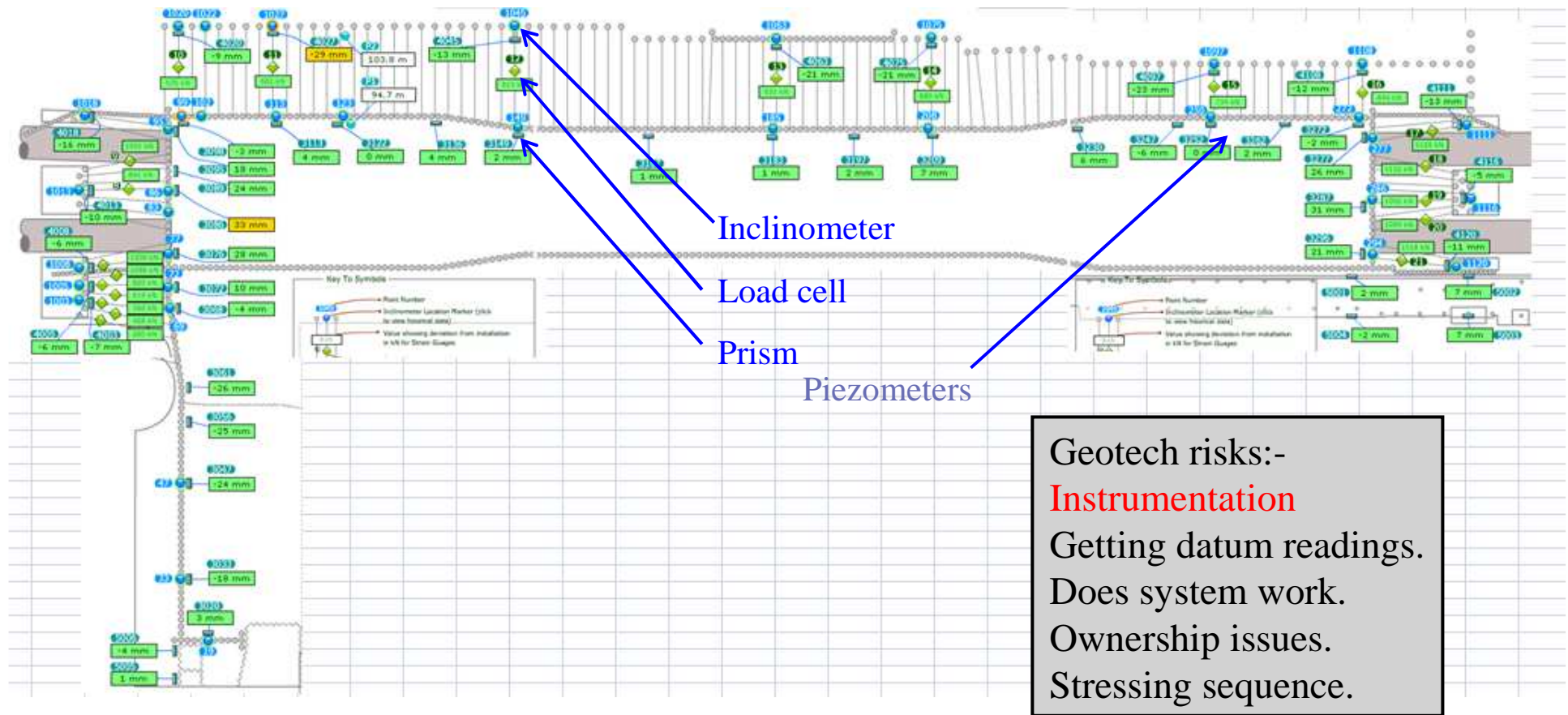
## 1.2m dia tubes with Crush Auger

Using reaction from 3 pre-installed casing to install the 4<sup>th</sup> casing

Chuck designed to extend to receive and push in the 5<sup>th</sup> casing partially



# Monitoring system



Geotech risks:-  
**Instrumentation**  
 Getting datum readings.  
 Does system work.  
 Ownership issues.  
 Stressing sequence.

- 19 full monitoring sections – inclinometers (manual), load cells and prisms (real time)
- Groundwater and dock water monitoring
- Web based access of monitoring data

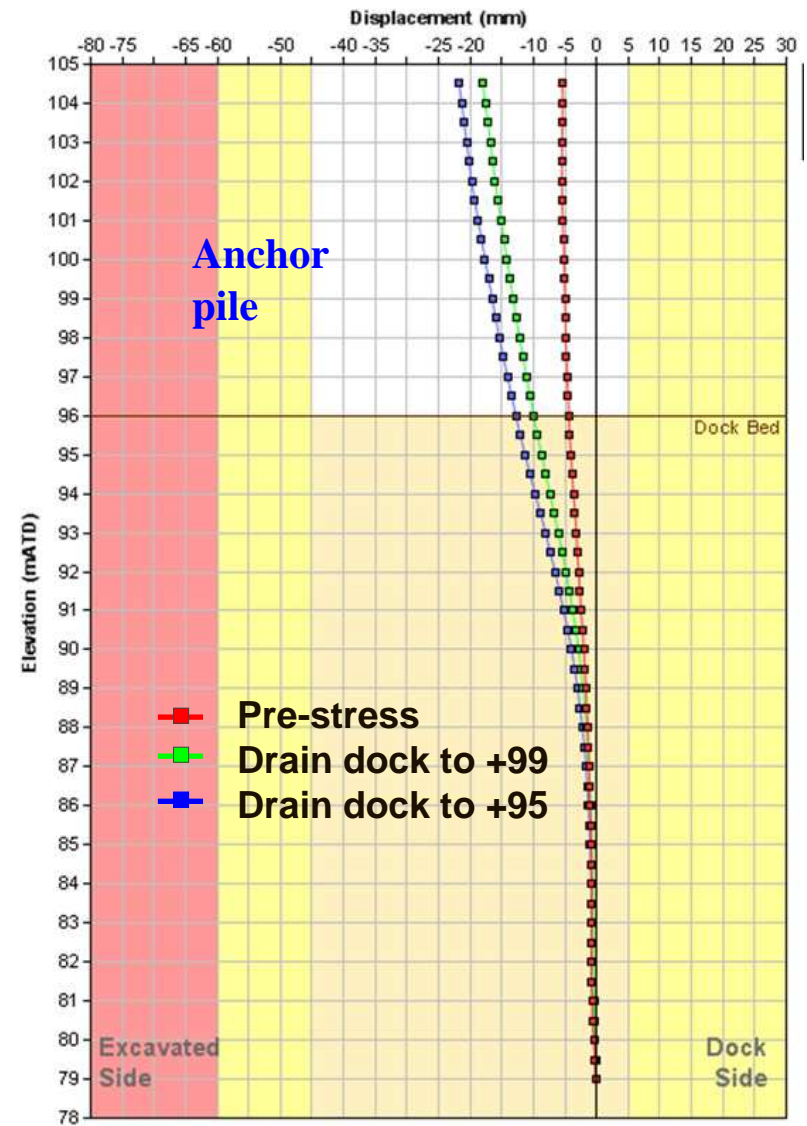
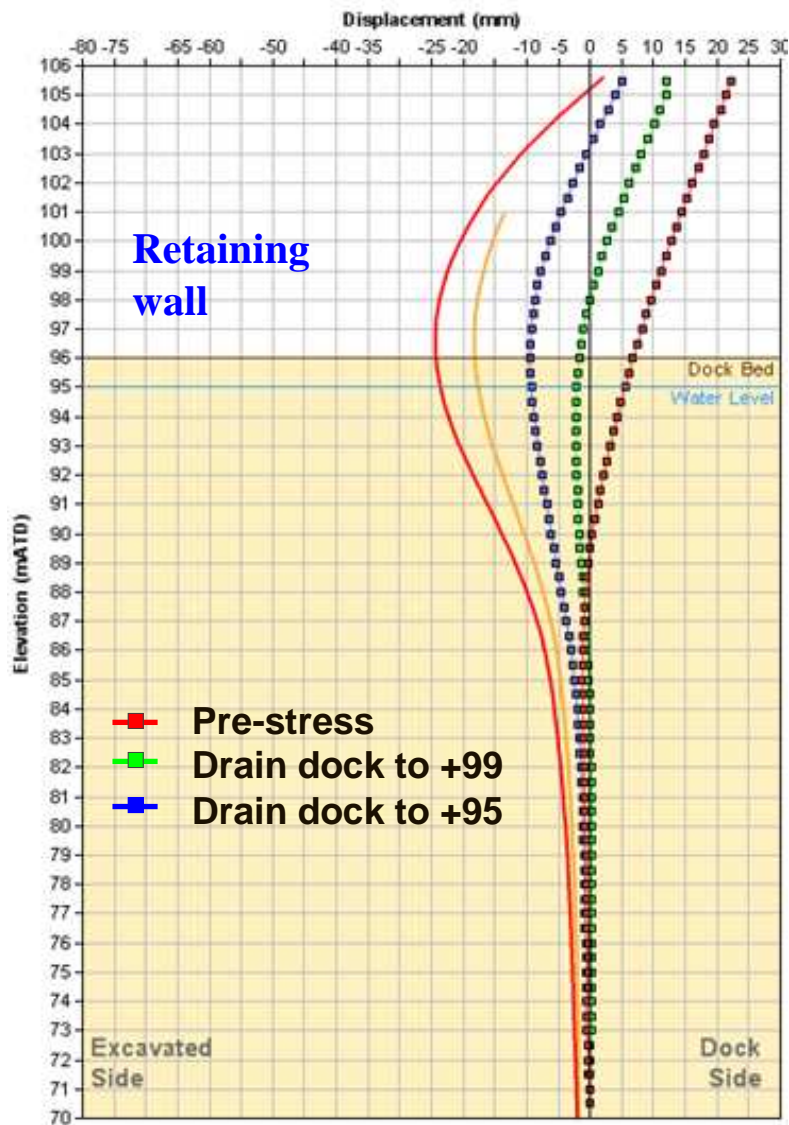


# Dock fully drained – mid March 2010



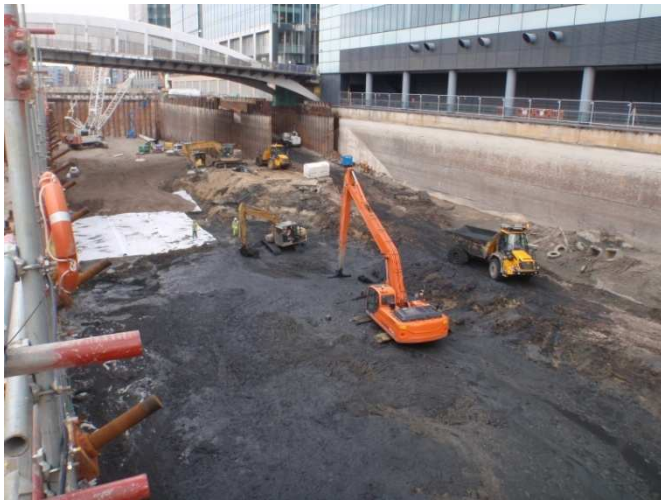
# Inclinometer readings – main wall and

# anchor pile





# Dock silt removal and secant piling



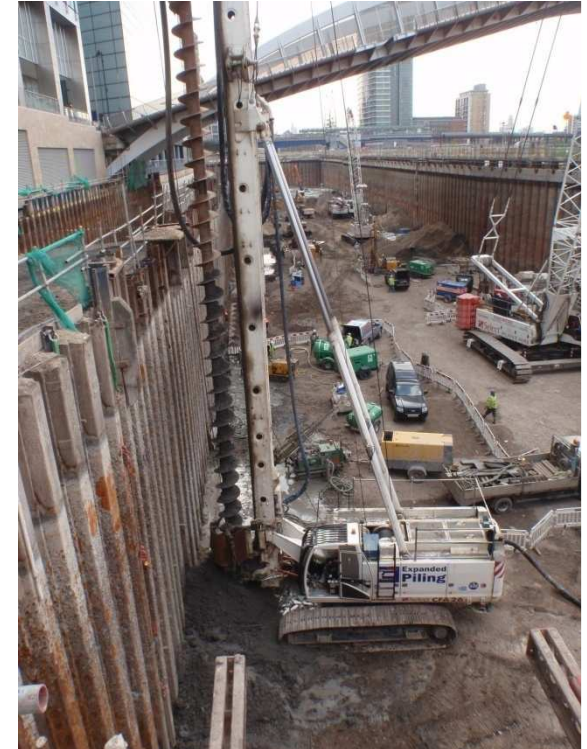
Dock silt removal – dig and dispose



Dock silt removal – wash and pump



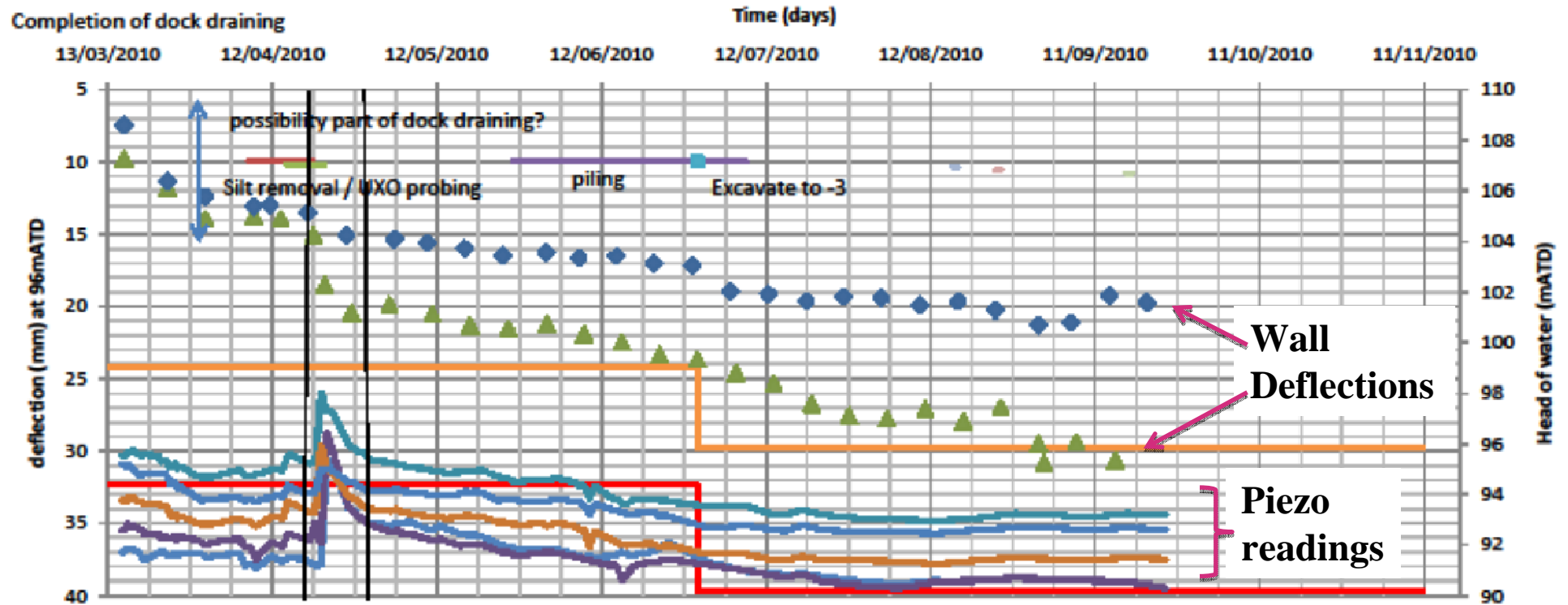
Guide wall installation



CFA piling of female soft pile

# Triggers Exceeded

Geotech risks:-  
 No allowance for UXO probe  
 Gradual “creep”



↔  
**UXO Probing**

Air flush rotary percussive  
 15m from wall or leakage

**Impact of UXO probing on wall deflection and piezometer readings**



# Plunge column installation

## Column tolerances

- +/- 25mm in plan
- 1 in 400 verticality

## Pile tolerances

- +/- 25mm in plan
- 1 in 75 verticality



Precast guide hole for 2.1m pile



Plunge column guide frame installation



Plunge column installation  
(18m long, upto 27t)



# Level -3 slab nearing completion



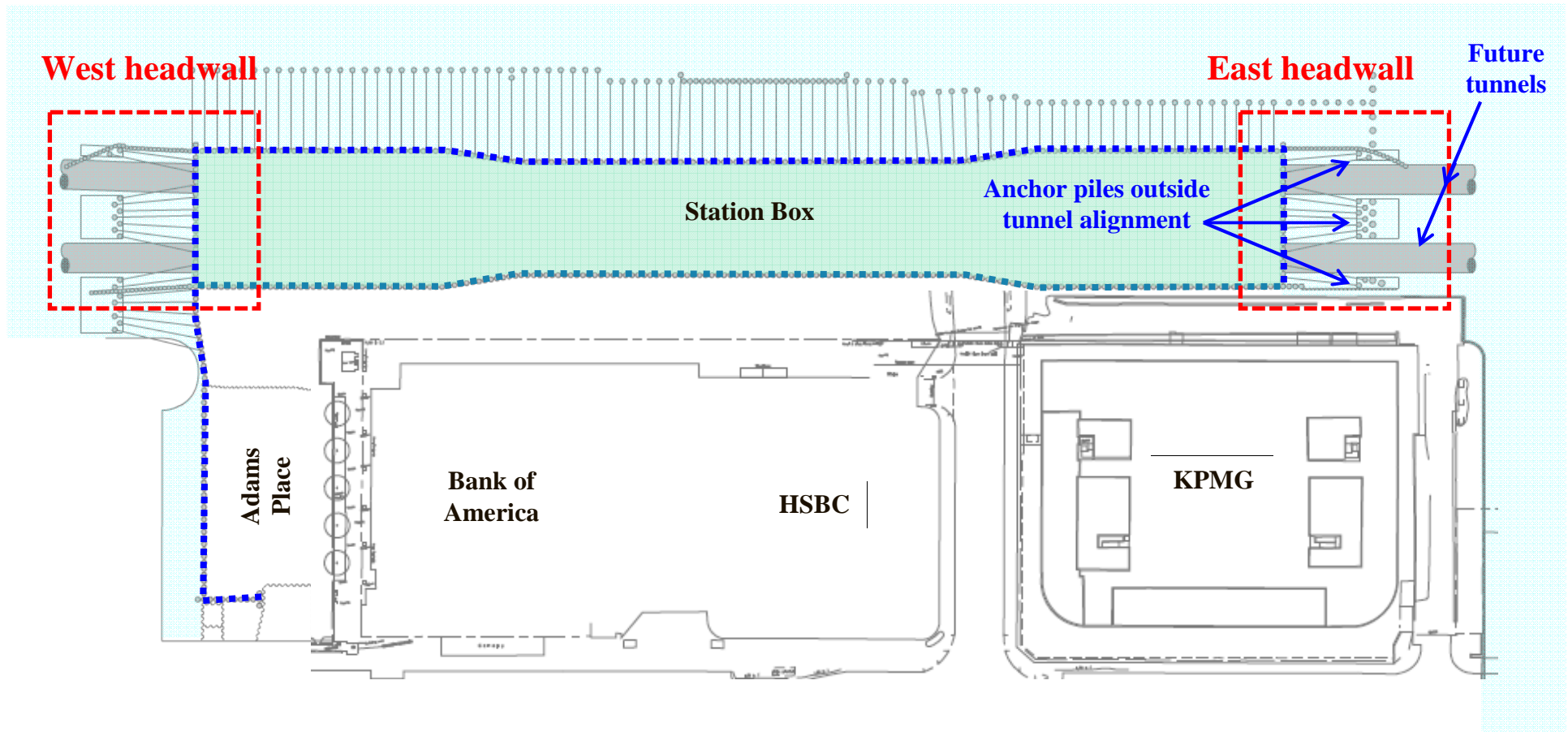


# Level -6 excavation, blinding, reinforcement etc



# OM – Best Way Out – Review and Modify soil parameters

- Tunnel alignment prevented evenly spread of anchor piles



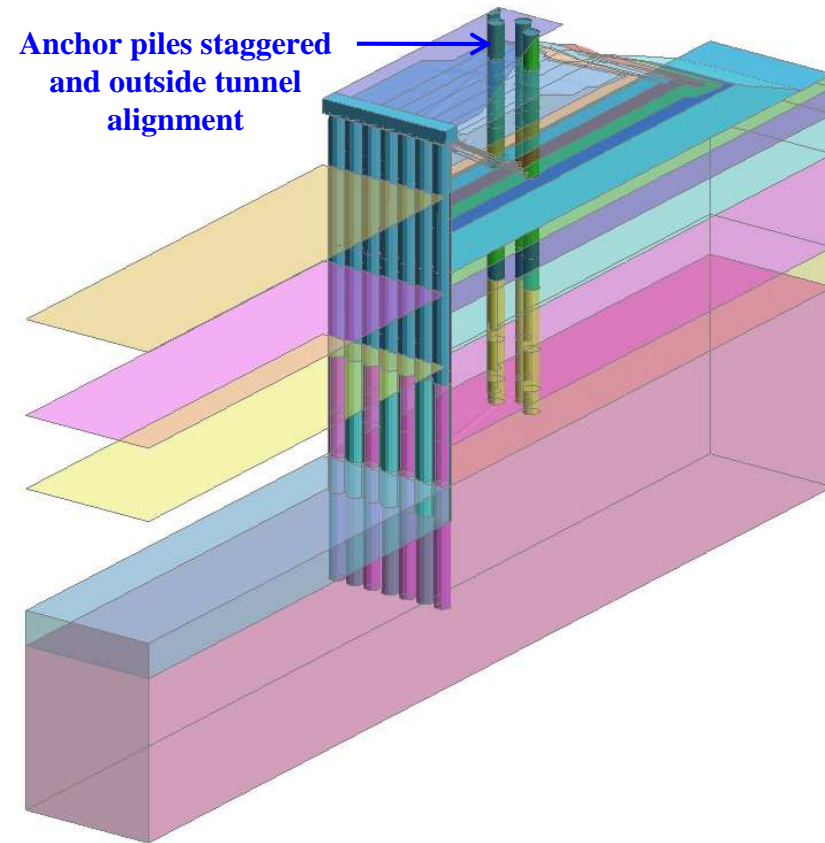
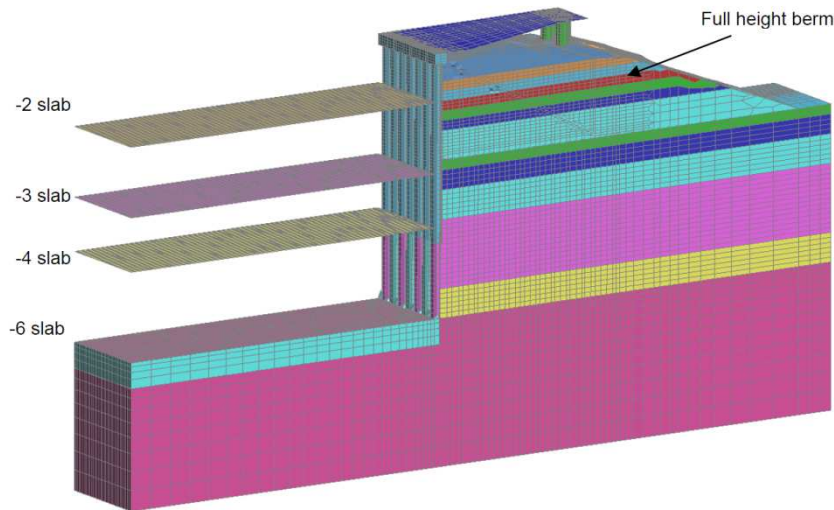
# Back analyse

## Review soil parameters

## Redesign - Remove berm and intermediate props

## OM – Best Way Out – New triggers

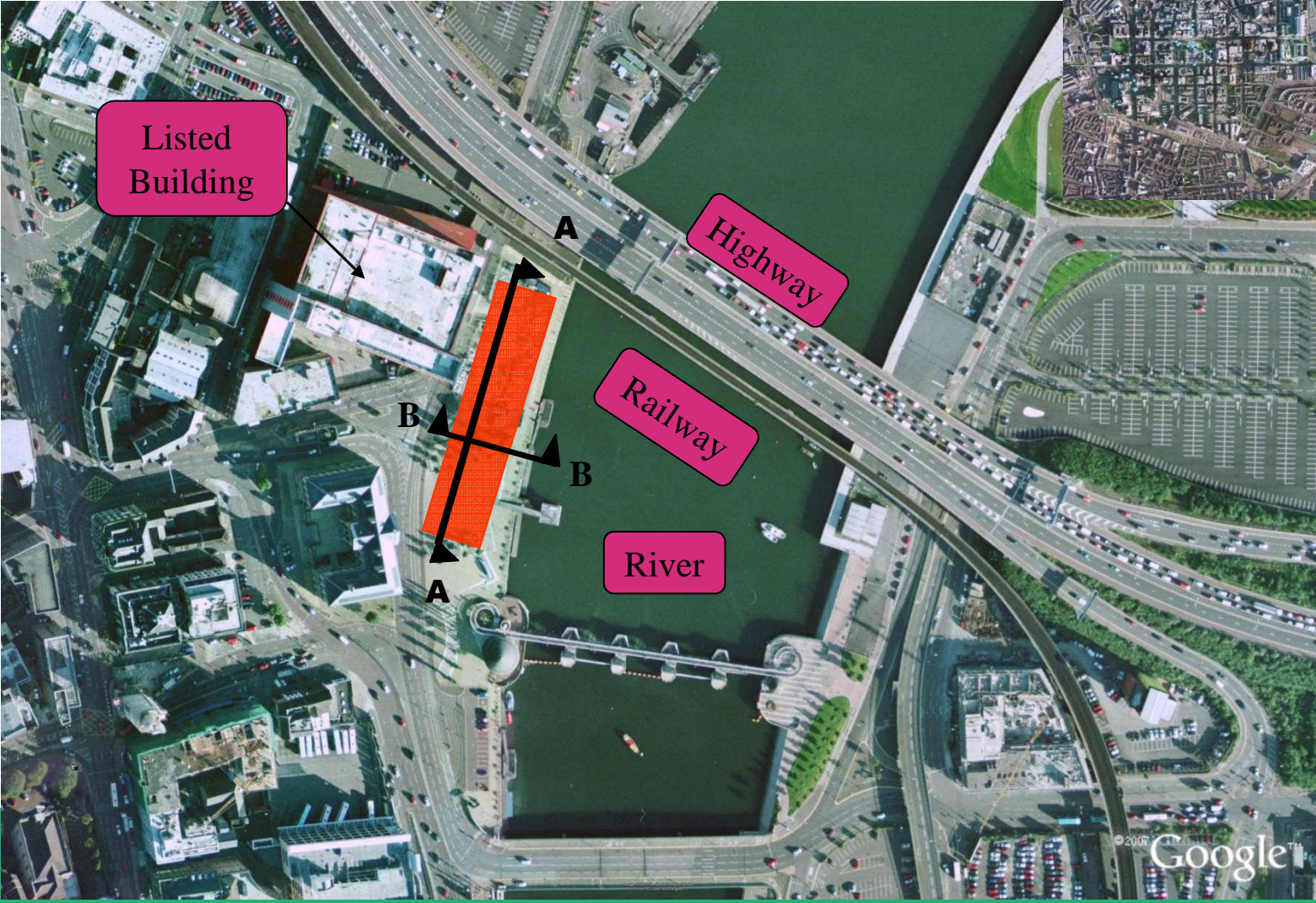
- The 2D simplified design approach verified using a 3D model when a revised construction sequence was proposed





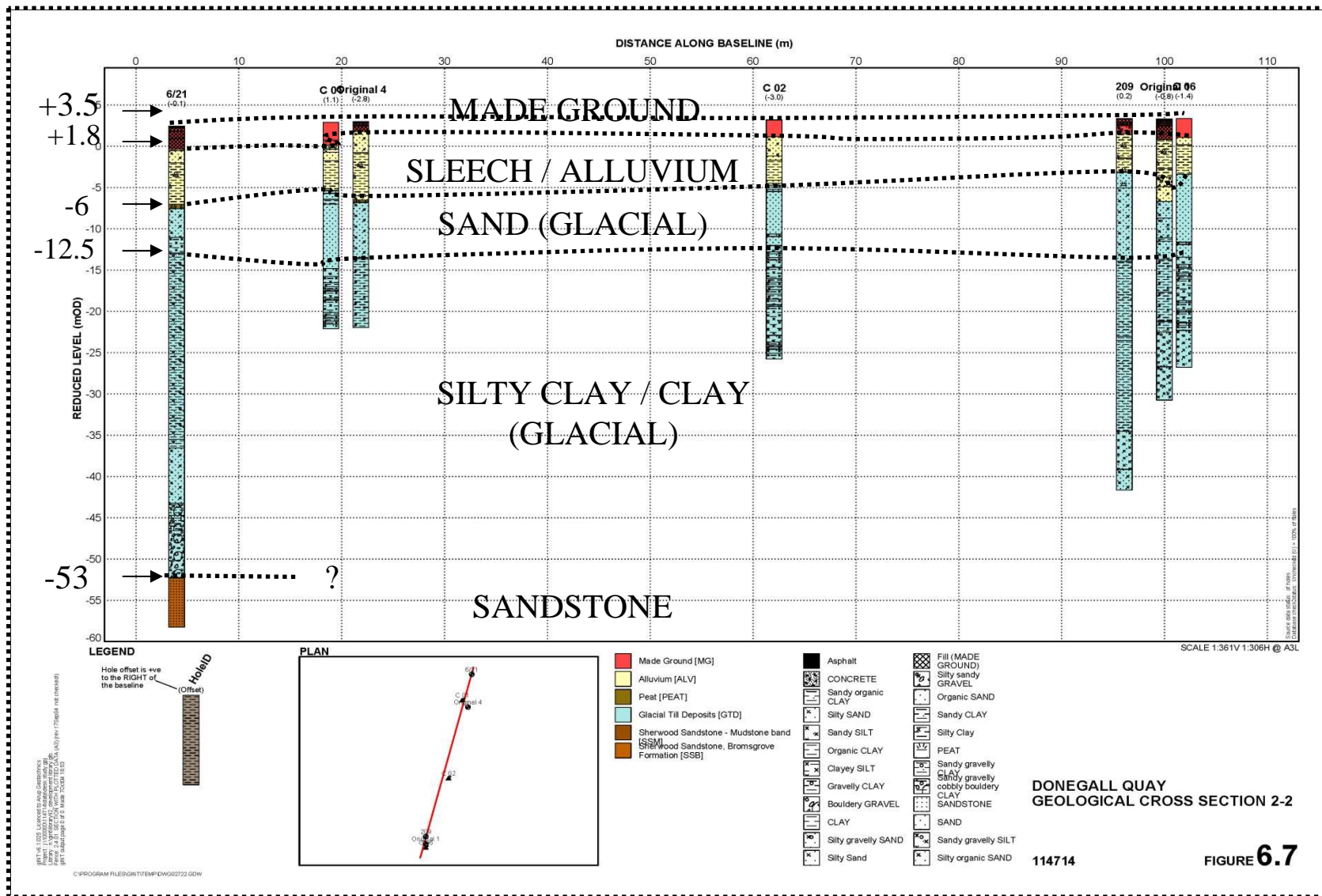
# Donegal Quay Development

Footprint Area ~ 35 m x 140 m



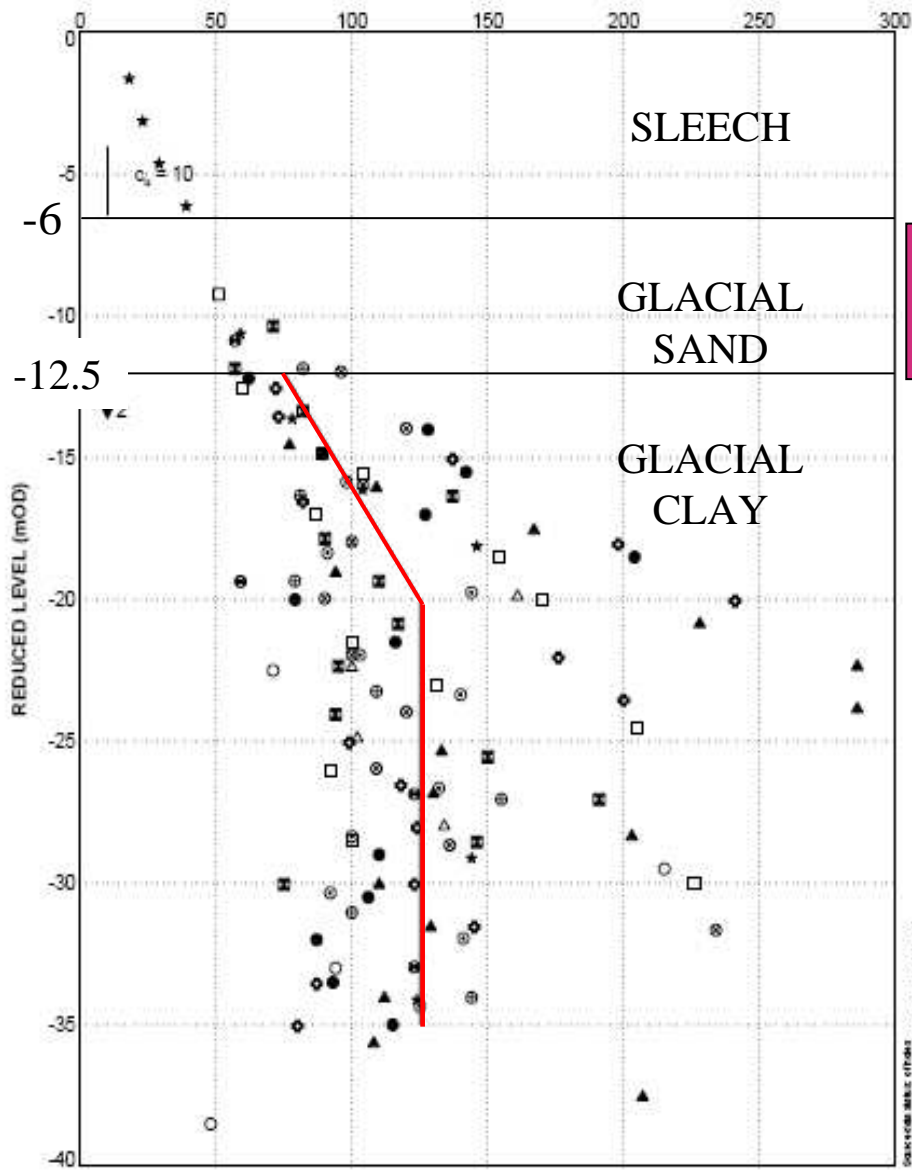


# Ground Conditions – Geotechnical Profile

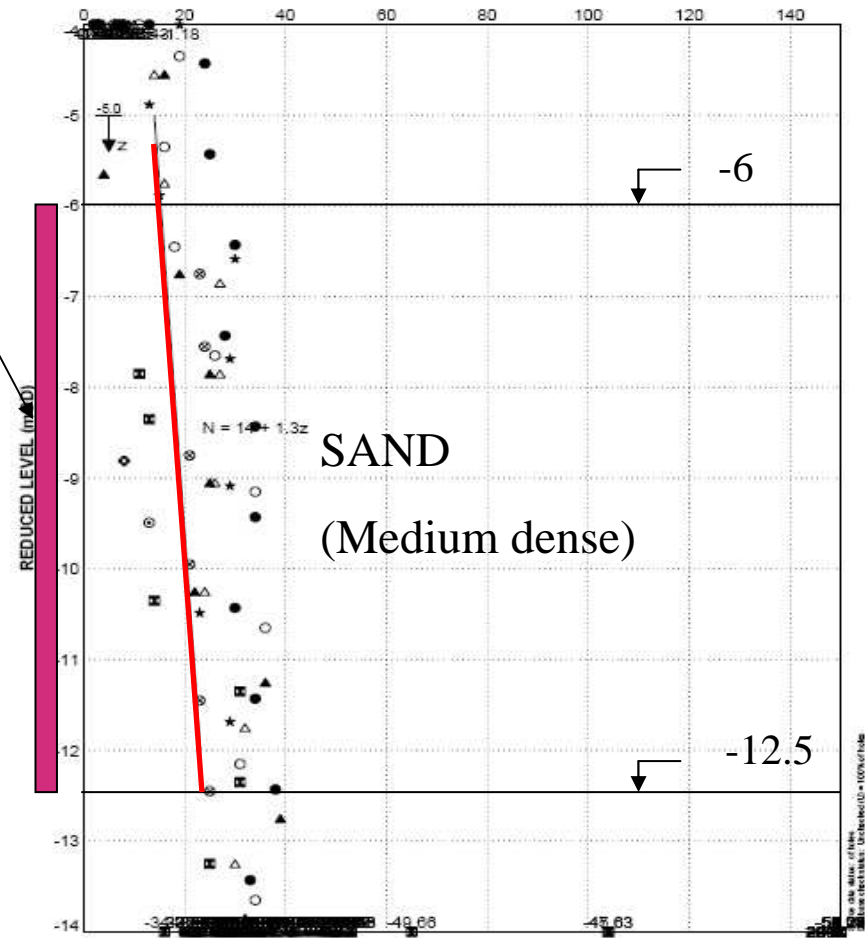


# Soil properties

## Undrained Strength (kPa)



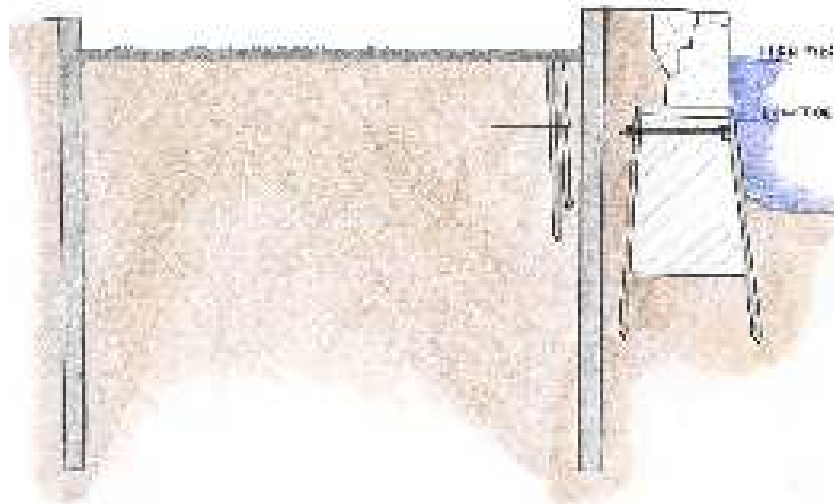
## SPT Profile



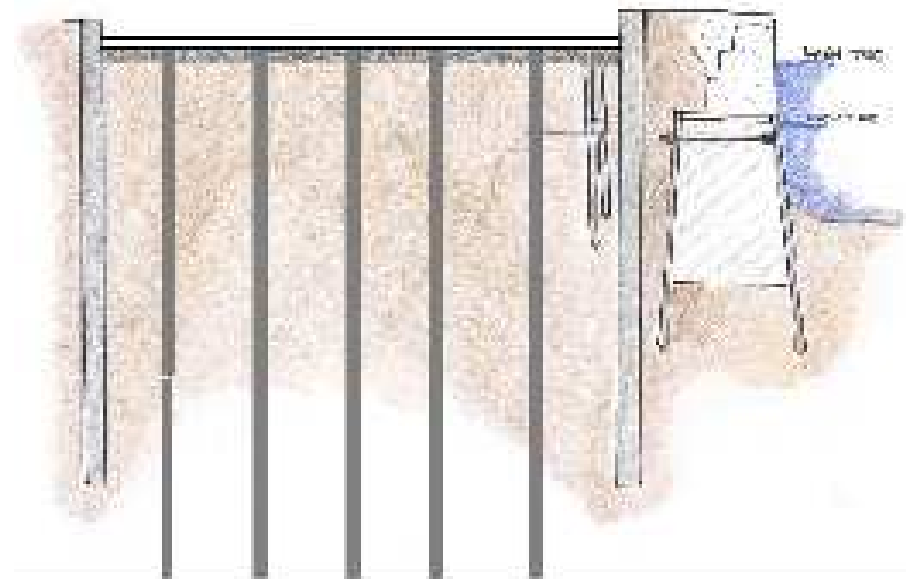
# Construction Sequence

- **Stage 1:** Site preparation.
- **Stage 2:** Install Sheet Pile walls.
- **Stage 3:** 2.0m excavate – remove obstruction / timber piles
- **Stage 4:** Install CFA pile approx 27m deep.

Stage 3

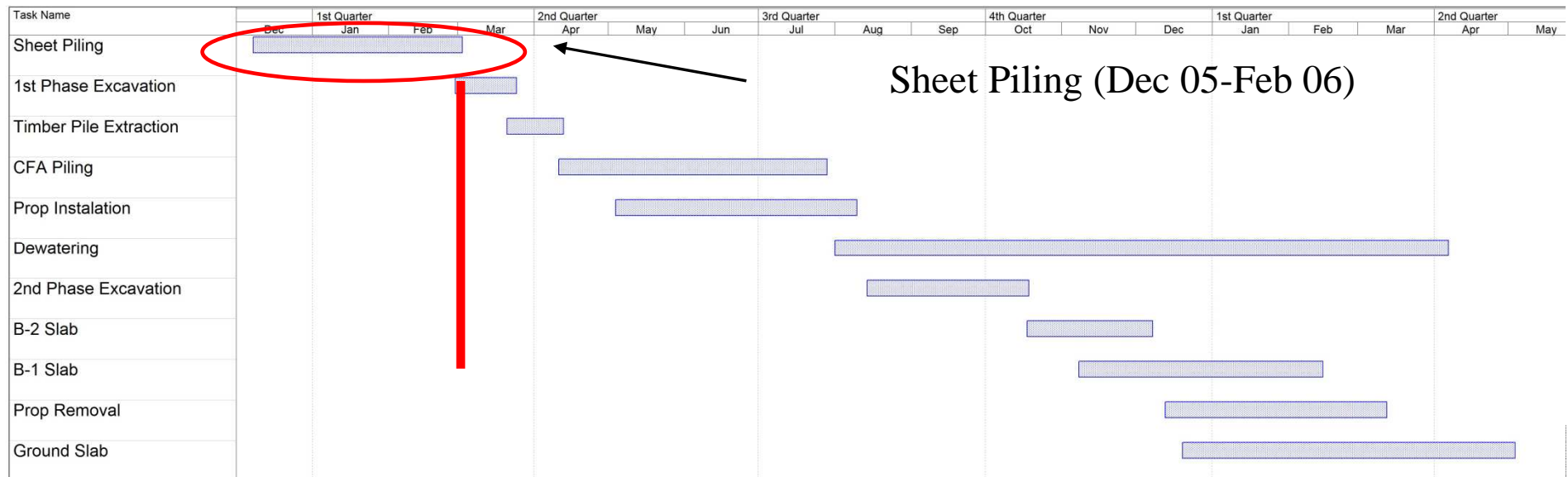


Stage 4

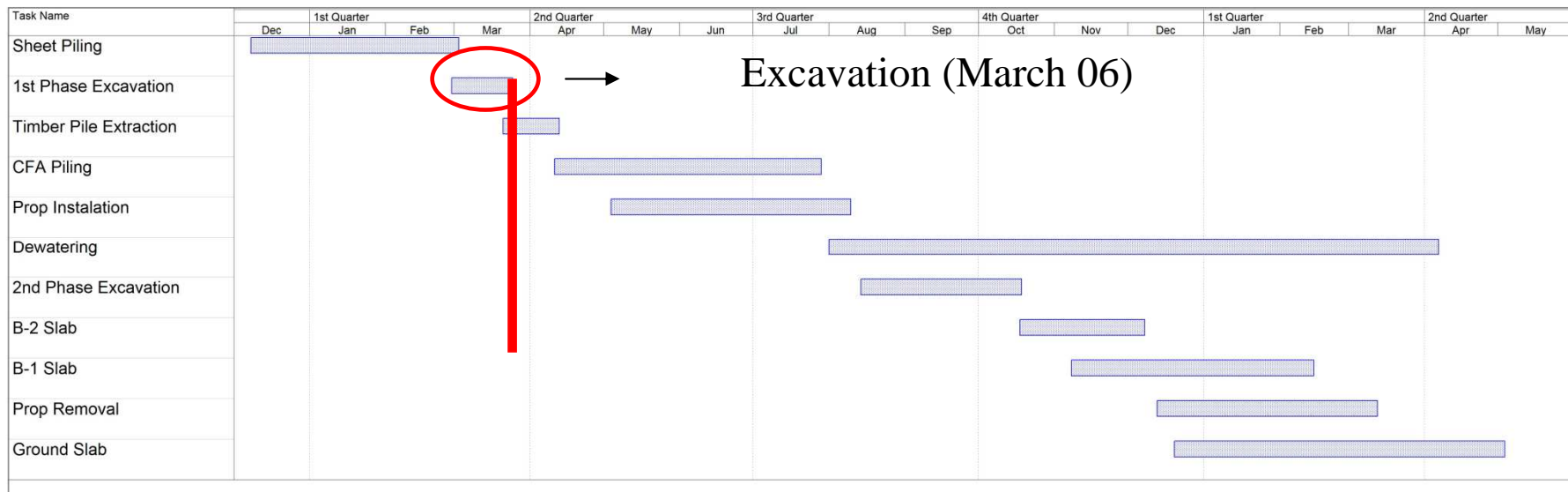




# Sheet Pile Installation

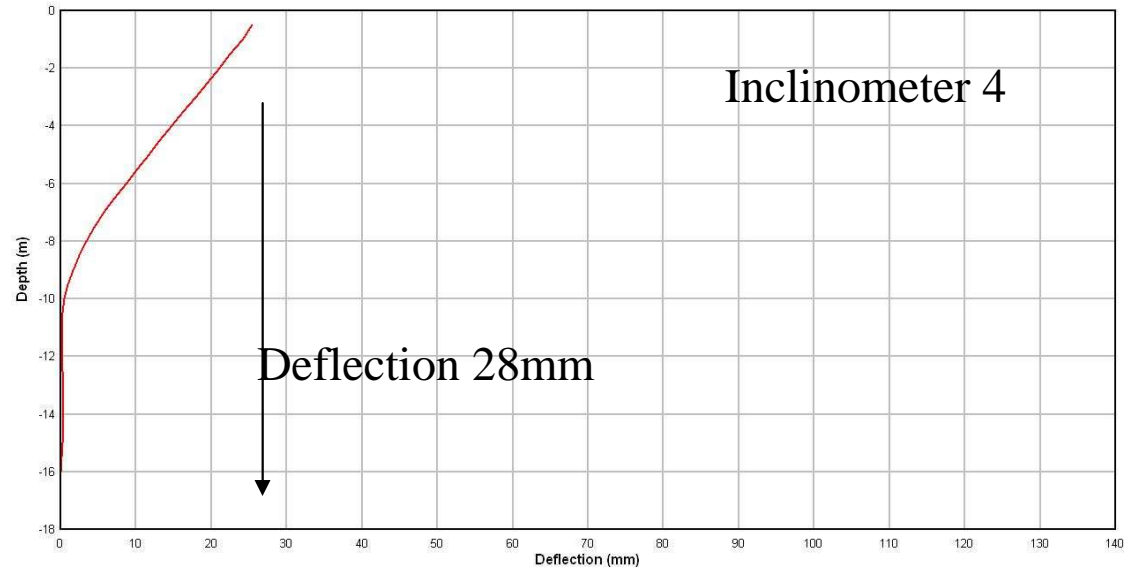


# Phase 1 Excavation to +1.5 mOD

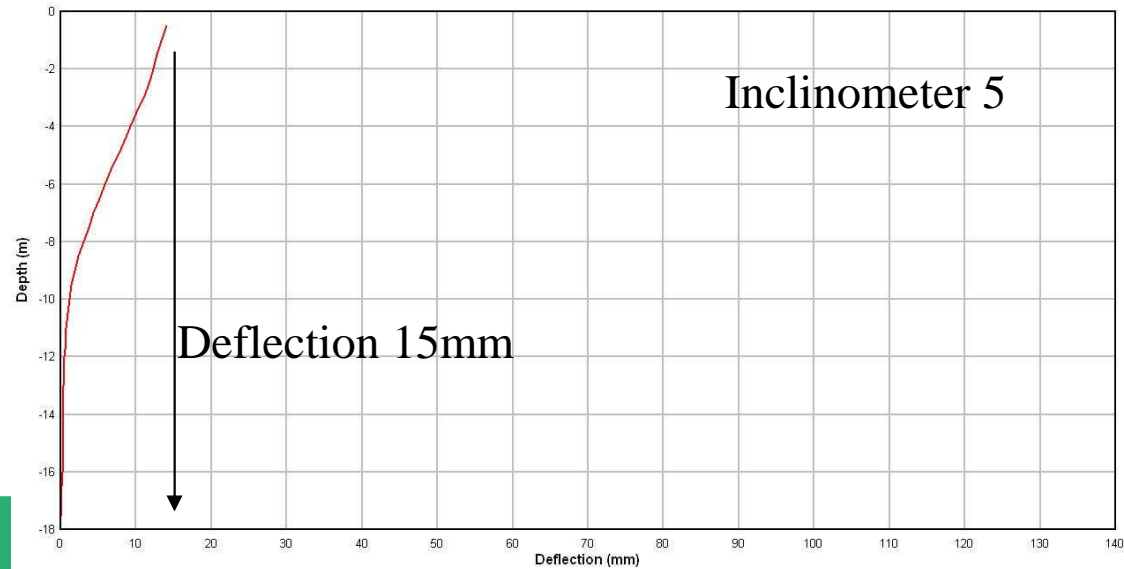


# Inclinometers 14 March 2006 - Cantilever dig

Inclinometer 4  
14/03/2006

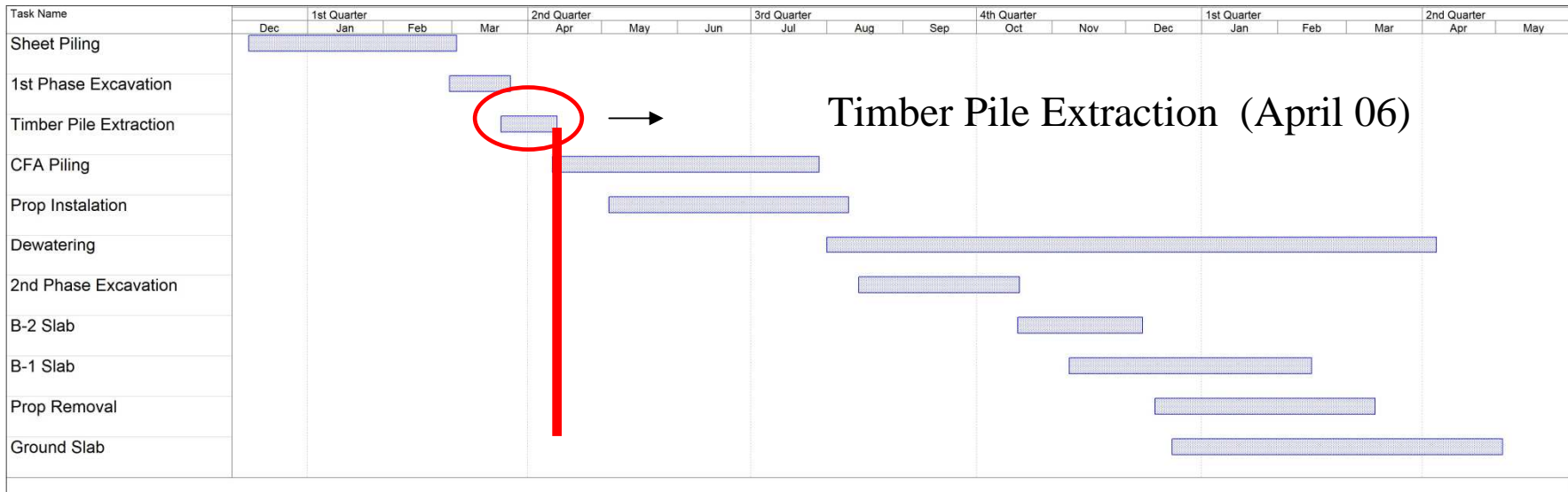


Inclinometer 5  
14/03/2006

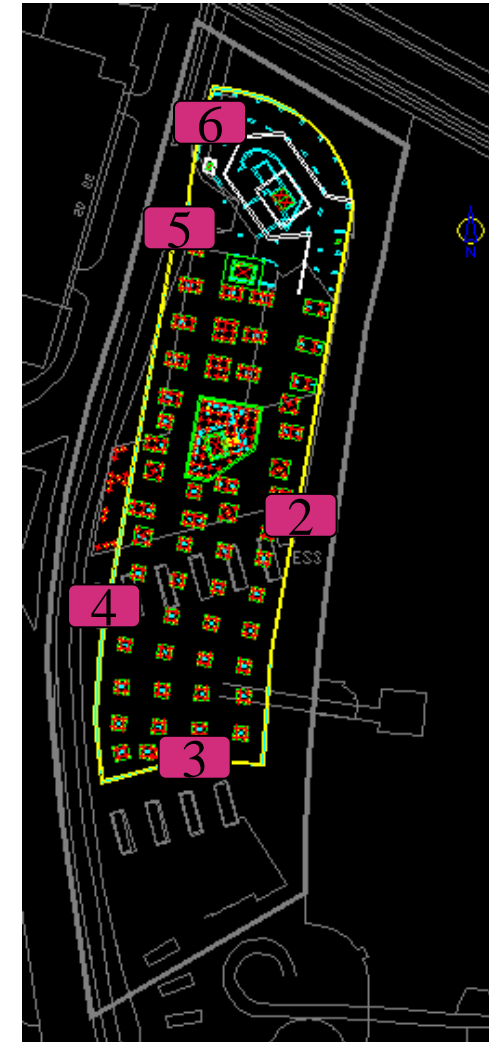
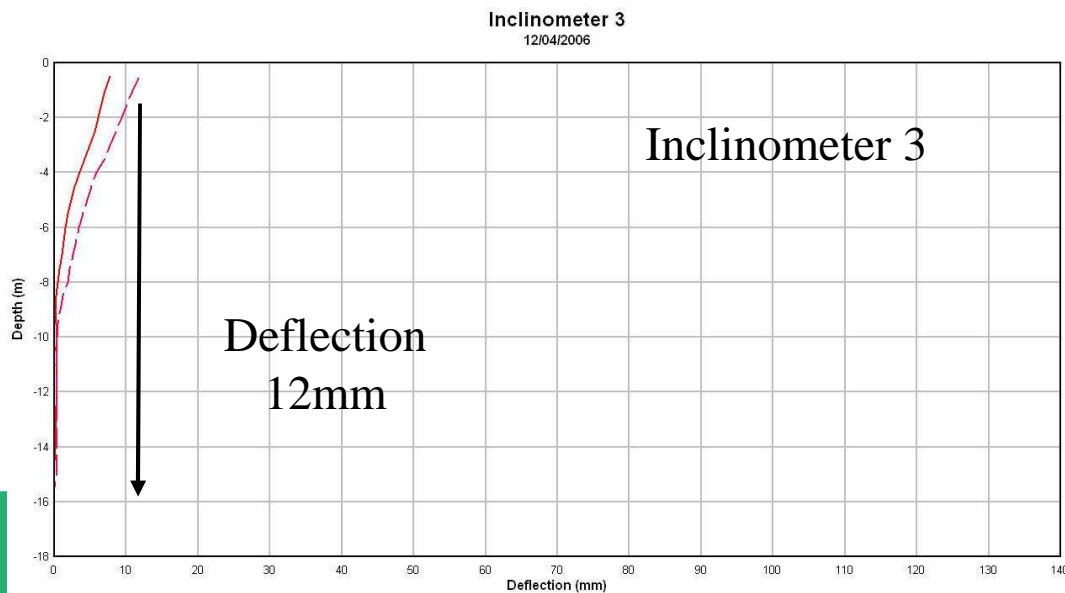
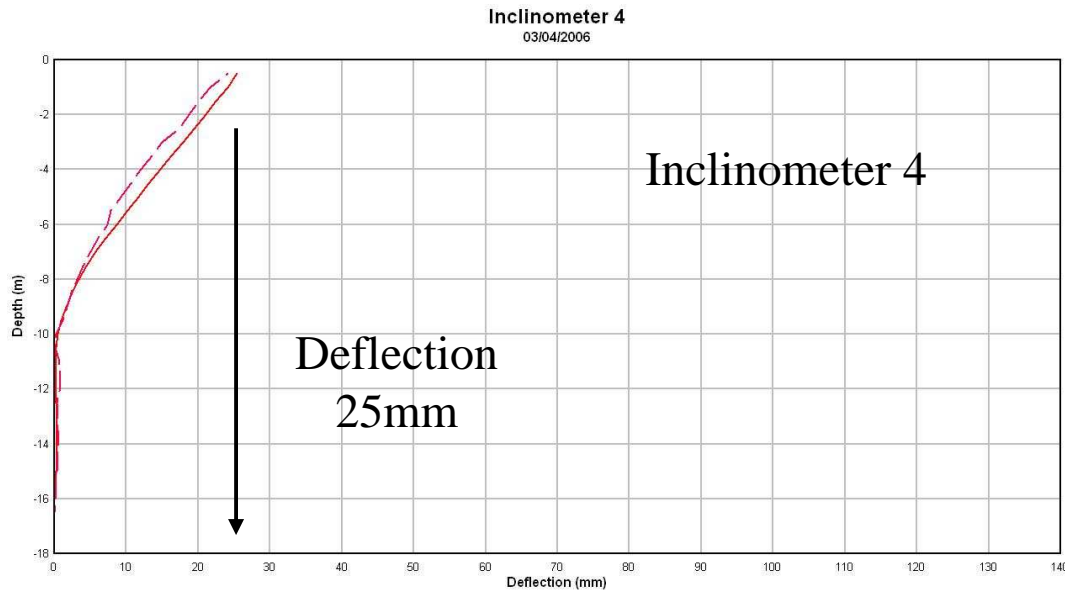




# Timber Pile Extraction

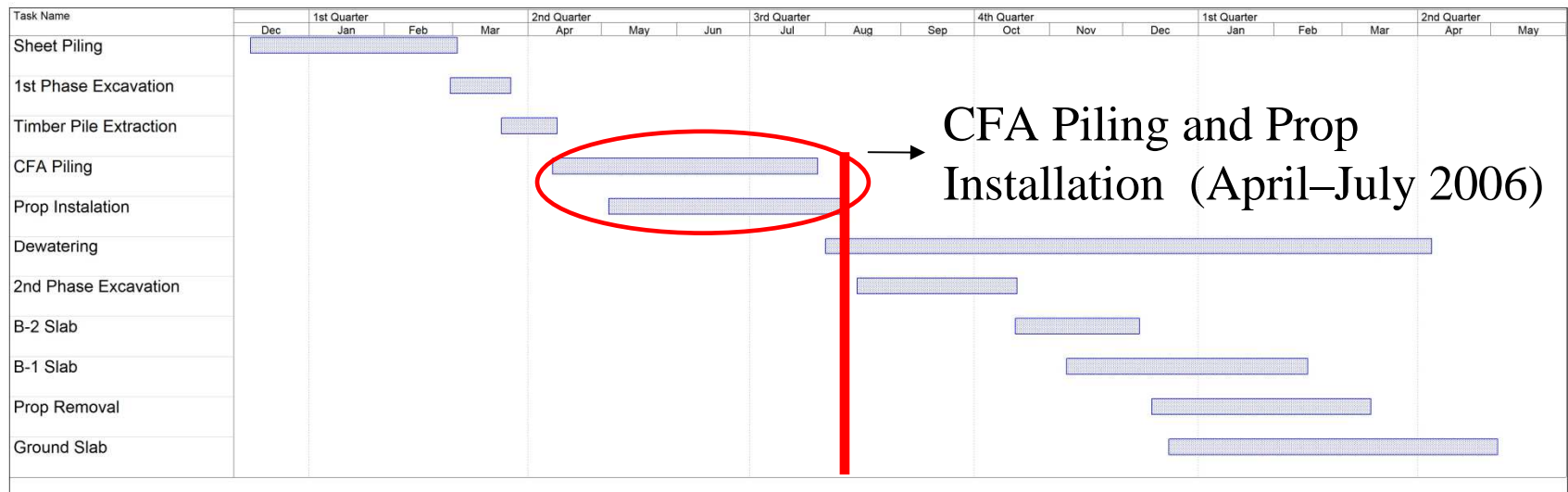


# Inclinometers 12 April 2006 - Timber Pile Extraction



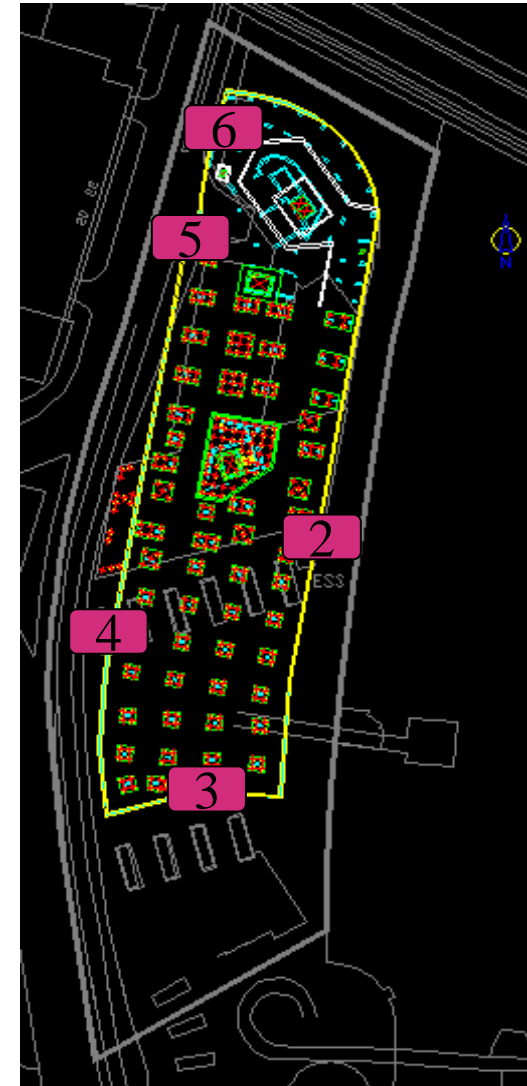
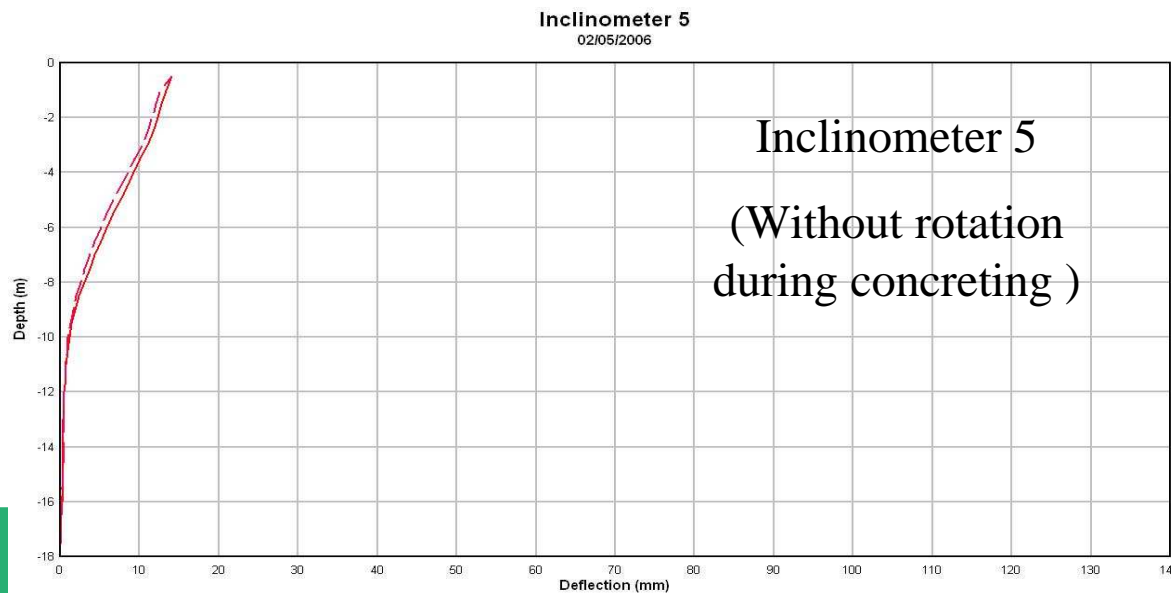
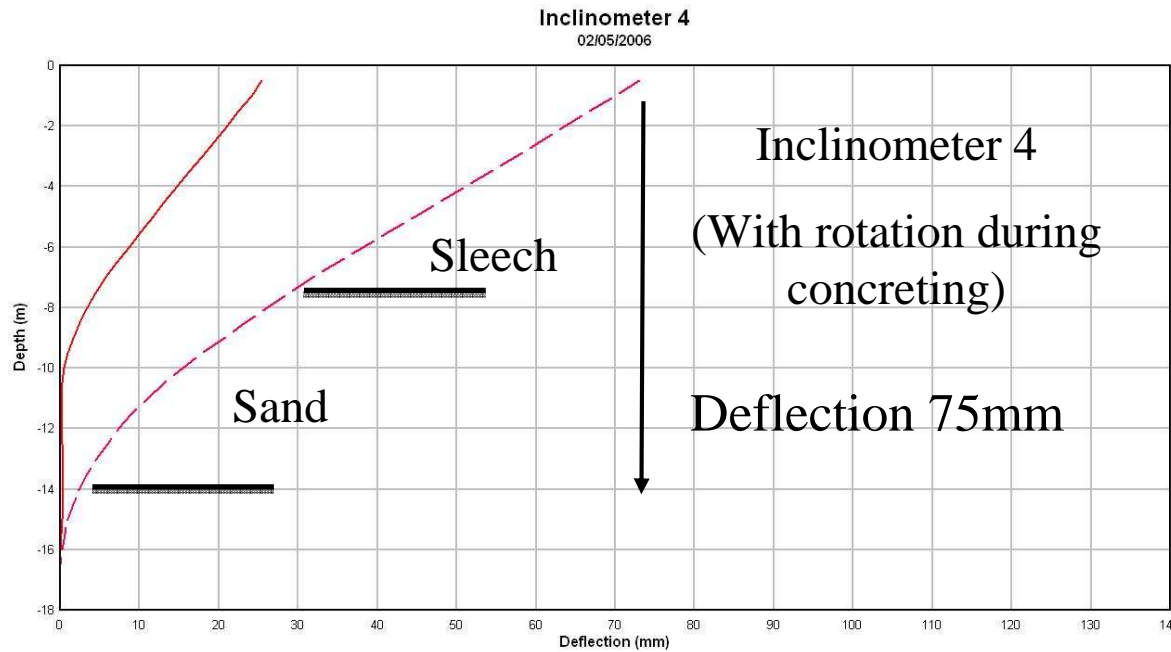


# CFA Piling Works and Prop Installation



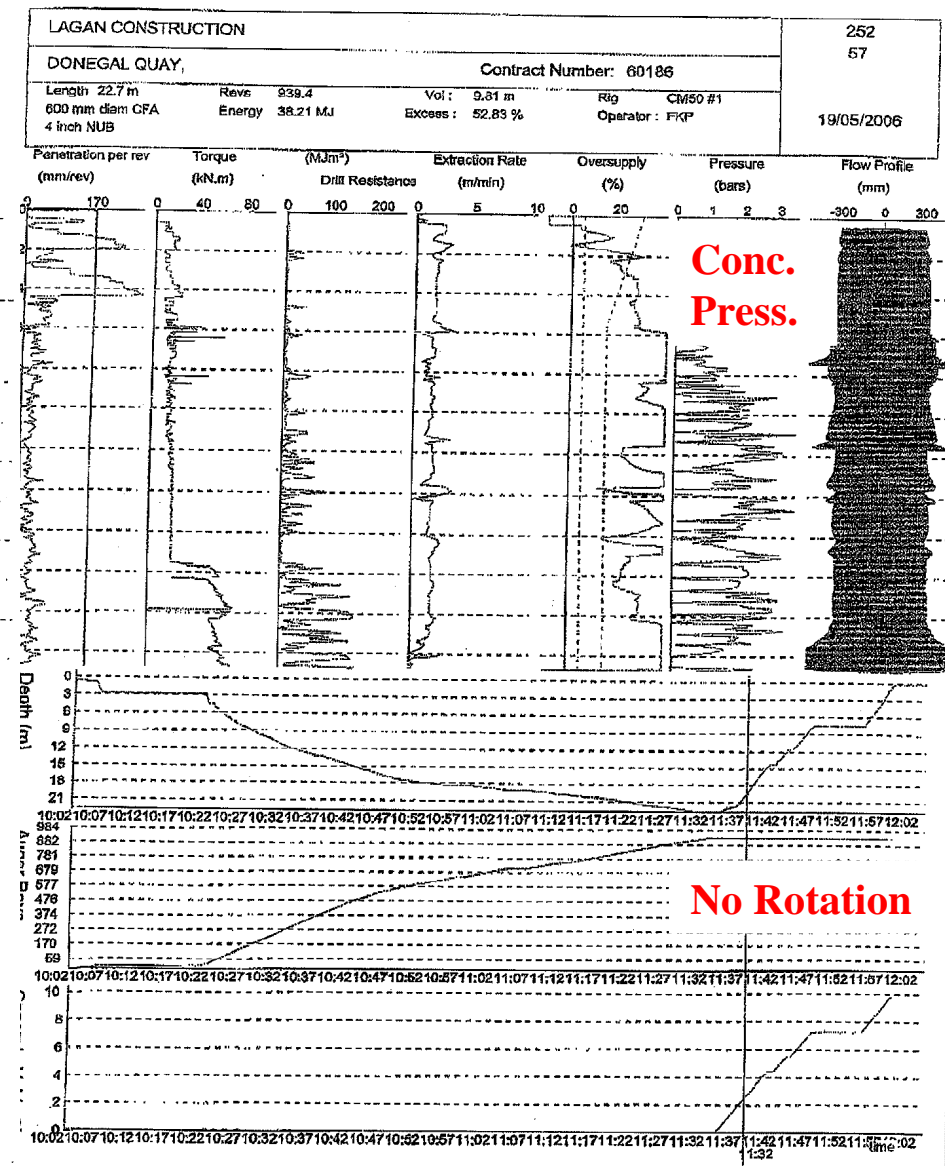
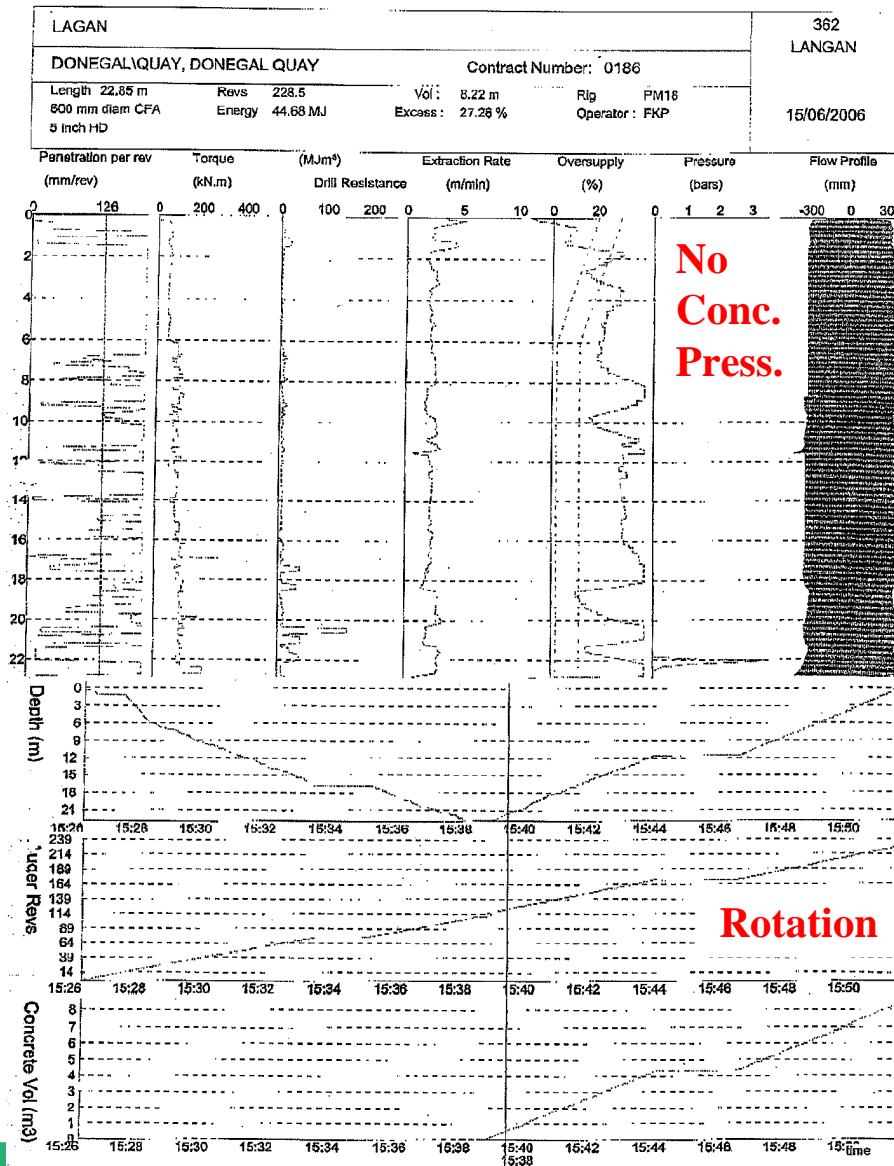


# Inclinometers - 02 May 2006 - CFA Flighting of Sleece



# Pile 382 – Concreting Revs - 4 rev/m

# Pile 252 - No rotation during concreting



## Over rotation and flighting - soft clays and loose Sand - interbedded soils

Minimise flighting

Maximises concrete pressures

Meet EN1536 - Cl 8.4.6.5

Use powered auger cleaner enables auger  
to be extracted safely without rotation

Used on all soil types

Auger diameters 300mm – 2000mm

About 1m reduction to drilling depth

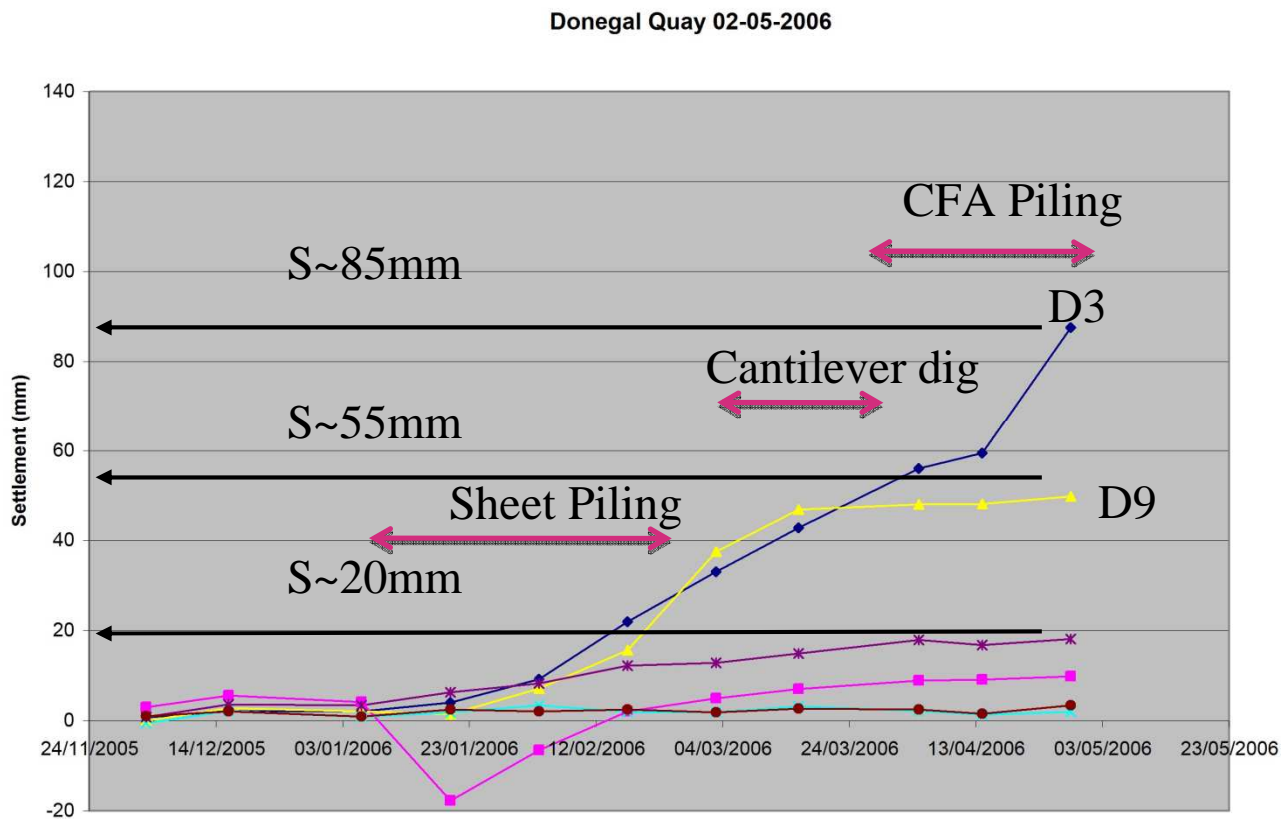
**Alternatively - Use cased CFA**



*Dawson Construction Plant Ltd*

# Settlements

## Sheet piles/ Dig / CFA piling



Trigger set at 50mm for whole construct!!

Review – What was causing movement

- Reassessment of trigger values – impact on utilities / buildings

# Donegall Quay Comments

Construction processes cause ground movements

- Wall installation
- Pile installation
- Anchor installation



Specify limits and incorporate into movement calculations

- Amber trigger= 3mm
- Red trigger= 5mm

These movements occur rapidly and continuous monitoring required until process is checked!!



# Conclusions

- Peck (1969) set out the Principles of OM
  - “Ab Initio” and “Best Way Out”
- Ciria (1999) R185 considers only the Ab Initio approach.
- Develop use of Conventional design – review - best way out
- **Kings Place** – reassessment of triggers set by adjacent buildings
- **Nicoll Highway collapse** - Not OM – Lessons on back analysis and redesign processes.
- **Canary Wharf Crossrail Station** - Use of Review Back Analysis and Best Way Out
- **Donegall Quay** – Impact of wall / pile / Anchor installation effects



**Thank you for your attention.**

**Any Questions?**